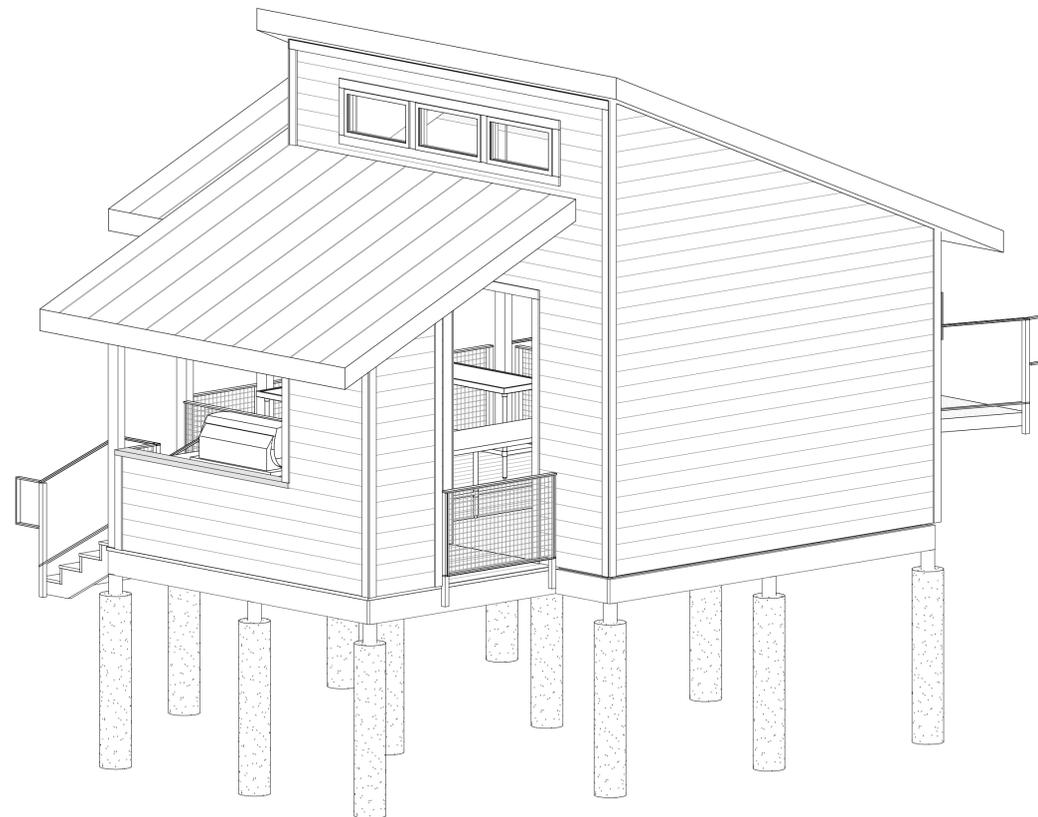


3D VIEW - SOUTHEAST - NEIGHBORHOOD 8 **2**



3D VIEW - NORTHEAST - NEIGHBORHOOD 8 **1**

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Mobile Loaves & Fishes

**Community
First! Village -
Kitchens -
Phase 3 -
Neighborhoods
8 & 9**

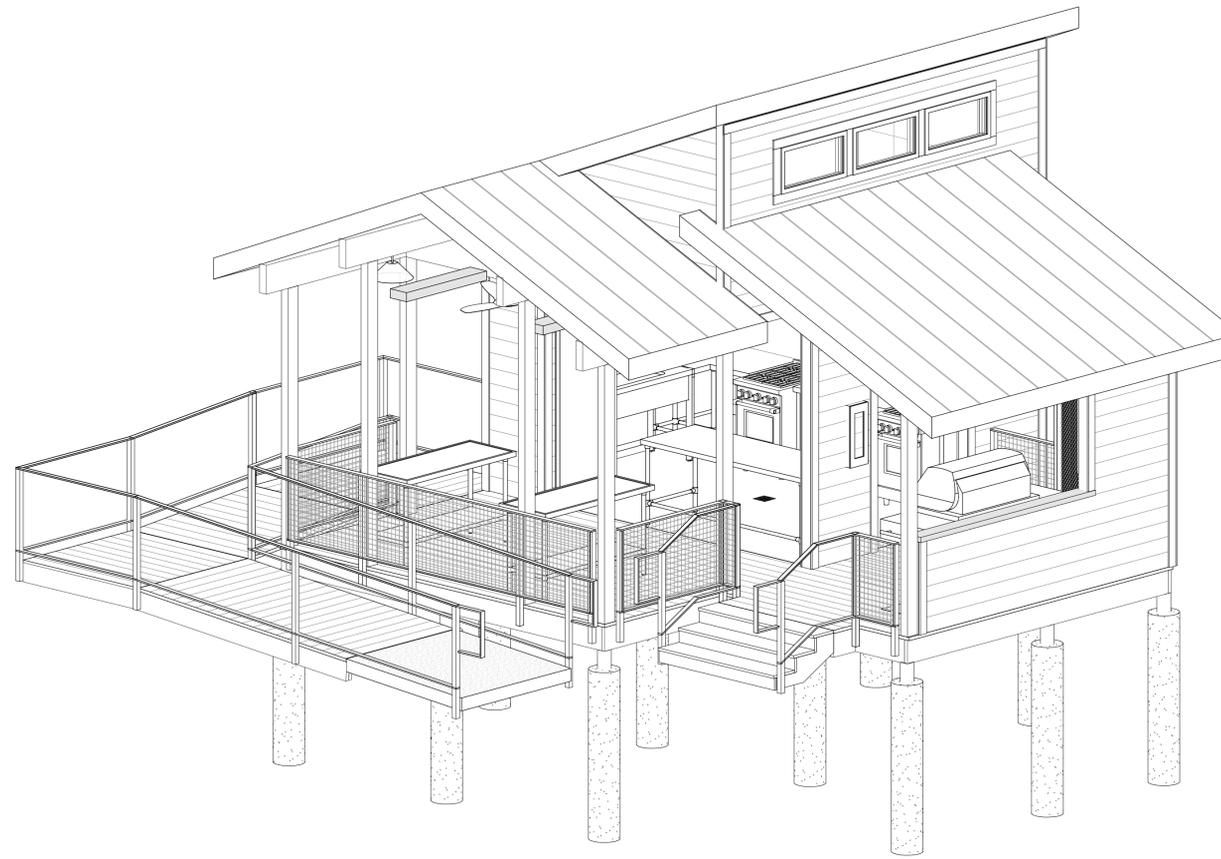
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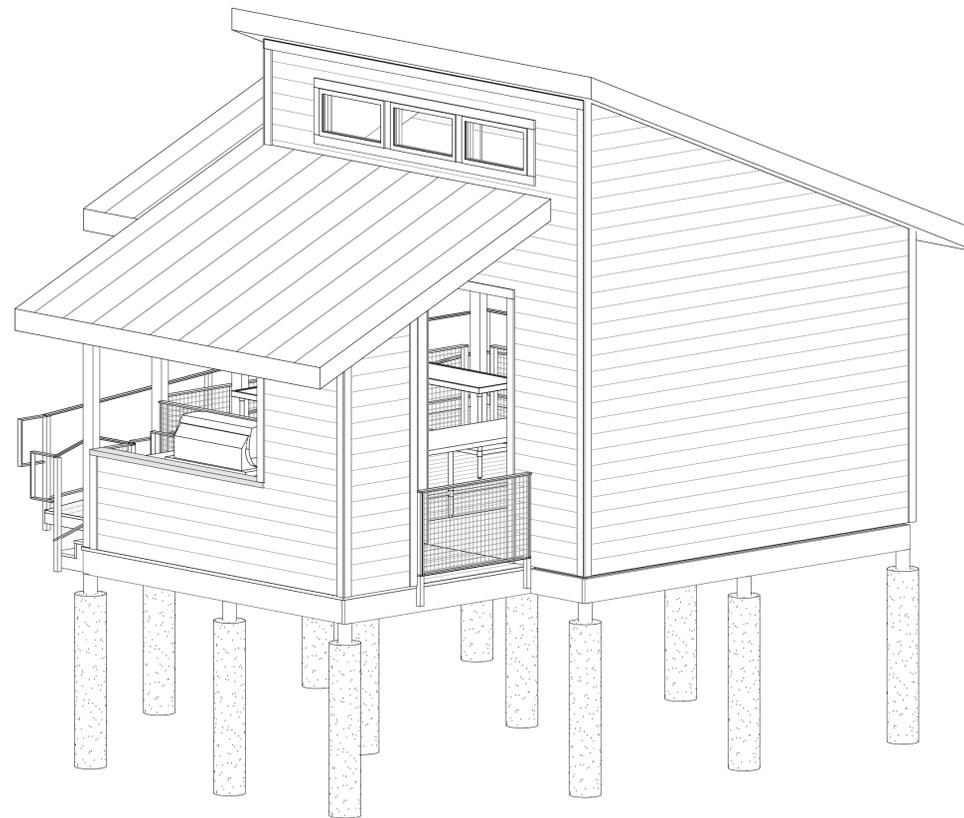
Project Number: 24-093a

3-D VIEWS -
NEIGHBORHOOD 8

A010



3D VIEW - SOUTHEAST - NEIGHBORHOOD 9 **2**



3D VIEW - NORTHEAST - NEIGHBORHOOD 9 **1**

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3-D VIEWS -
NEIGHBORHOOD 9

A011

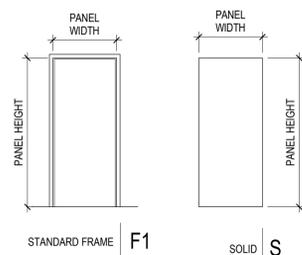
DOOR SCHEDULE - EXTERIOR (ALL LEVELS)											
NO.	PANELS				FRAMES			HARDWARE			COMMENTS
	PANEL TYPE	PANEL WIDTH	PANEL HEIGHT	PANEL MATERIAL	PANEL FINISH	FRAME TYPE	FRAME MATERIAL	FRAME FINISH	CARD READER	HARDWARE	
CL	S	2'-0"	7'-0"	HM	PTX	F1	HM	PTX	YES	C201C	

TOILET ACCESSORIES SCHEDULE			
MARK	DESCRIPTION	MANUFACTURER	MODEL
TA1	RECESSED SOAP DISPENSER WITH SOAP VESSEL	BOBRICK	B-306

Hardware Group No. C201C
For use on Door #6:

QTY	DESCRIPTION	CATALOG NUMBER	FINISH	MFR
3	EA HINGE	58B1 4.5 X 4.5	IVE	IVE
1	EA POWER TRANSFER	EPT10	889	VON
1	EA EU MORTISE LOCK	L3002HDEU 18A RX CON 12/24	628	SCH
	VDC			
1	EA SFC CORE	C067	626	FAL
1	EA SURFACE CLOSER WITH HOLD OPEN ARM	4040XP SHCUSH	689	LCN
1	EA KICK PLATE	8400 10" X 2" LDW B-CS	630	IVE
3	EA SILENCER	S164	GRY	IVE
1	EA CREDENTIAL READER BY SECURITY - DIVISION 28			BIO
1	EA DOOR POSITION SWITCH BY SECURITY - DIVISION 28			
1	EA POWER SUPPLY BY SECURITY - DIVISION 28			

DOOR NORMALLY CLOSED AND LOCKED
INGRESS BY THE CREDENTIAL READER OR KEY OVERRIDE.
FREE EGRESS AT ALL TIMES.



GENERAL DOOR, FRAME AND HARDWARE NOTES

- ALL HANDLES, PULLS, LATCHES, LOCKS AND OTHER OPERATION DEVICES ON ALL ACCESSIBLE DOORS SHALL BE LEVER STYLE, U.N.O. AND MOUNTED AT 36" A.F.F.
 - ALL CLOSERS SHALL HAVE A SWEEP PERIOD ADJUSTED SO THAT FROM AN OPEN POSITION OF 70 DEGREES THE DOOR WILL TAKE AT LEAST 3 SECONDS TO MOVE TO A POINT 3" FROM THE LATCH.
 - ALL DOORS TO HAVE A MAXIMUM OF 5 LBS. OPENING FORCE.
 - COORDINATE KEYING WITH BUILDING OWNER.
 - ALL HOLLOW METAL AND RATED OPENINGS TO RECEIVE SEALS.
 - ALL EXTERIOR DOORS TO RECEIVE NEOPRENE OR RUBBER BULB GASKETS, NOT BRUSH SEALS.
 - REFER TO HARDWARE SPECIFICATIONS FOR ALL HARDWARE SETS.
 - THE GENERAL CONTRACTOR IS TO COORDINATE THE INSTALLATION OF CARD READERS, ELECTRONIC LOCKING DEVICES, AUTO OPERATORS, AND ALL OTHER MONITORING DEVICES WITH THE DOR AND FRAME HARDWARE.
- TEMPERED GLASS

KITCHEN NOTES:

- STAINLESS STEEL BASE CABINETS PROVIDED AND INSTALLED BY GC.
- FARM SINK INTEGRATED IN STAINLESS STEEL COUNTER, PROVIDED AND INSTALLED BY GC.
- EXHAUST HOODS PROVIDED AND INSTALLED BY GC.
- STORAGE LOCKERS PROVIDED BY OWNER AND INSTALLED BY GC.
- RANGES PROVIDED BY OWNER AND INSTALLED BY GC.
- WATER HEATER PROVIDED BY OWNER, INSTALLED BY GC.

GENERAL FINISH NOTES

- ALL FLOORING TRANSITIONS OCCURRING AT DOOR OPENINGS TO OCCUR AT CENTERLINE OF DOOR.
- PAINT ALL UNDER-COUNTER SUPPORTS TO MATCH ADJACENT WALL FINISH.
- WALL TEXTURE TO BE "LIGHT ROLLER STIPPLE".
- PROVIDE PAINT MANUFACTURER'S RECOMMENDED PRIMERS AND UNDERCOATS.

IBC 2021 CHAPTER 8 - INTERIOR FINISH REQUIREMENTS

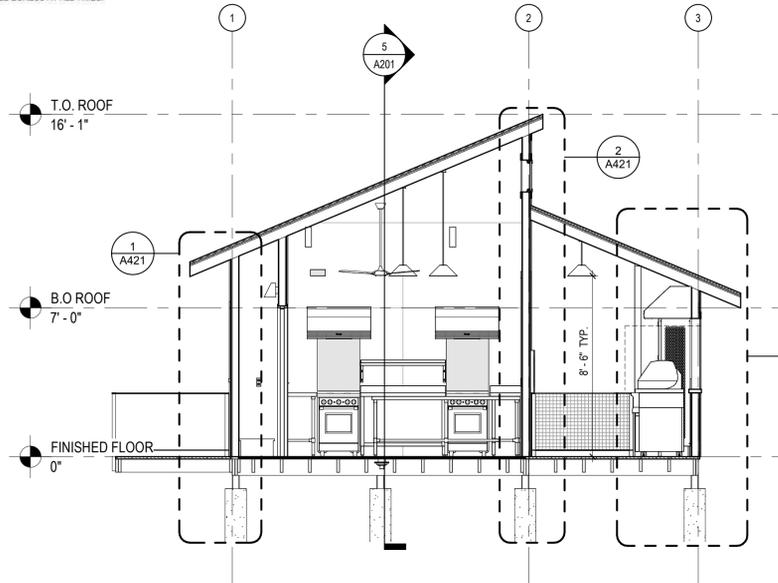
- THERE ARE NO COMBUSTIBLE MATERIALS USED IN BUILDING ELEMENTS, EXCEPT AS PERMITTED PER IBC 2021, SECTION 803.
- ALL MATERIALS IN THE PROJECT TO COMPLY WITH IBC 2021 SECTIONS 801 - 808.
- REFER TO FINISH KEY FOR FLAME SPREAD AND SMOKE-DEVELOPED INDEXES PER ASTM E 84 (IBC 2021, SECTION 803.1.2).
- REFER TO FINISH KEY FOR FLOORING RADIANT PANEL CLASS PER NFPA 253 OR ASTM E 648. (IBC 2021, SECTIONS 804.2).

GENERAL PLAN NOTES

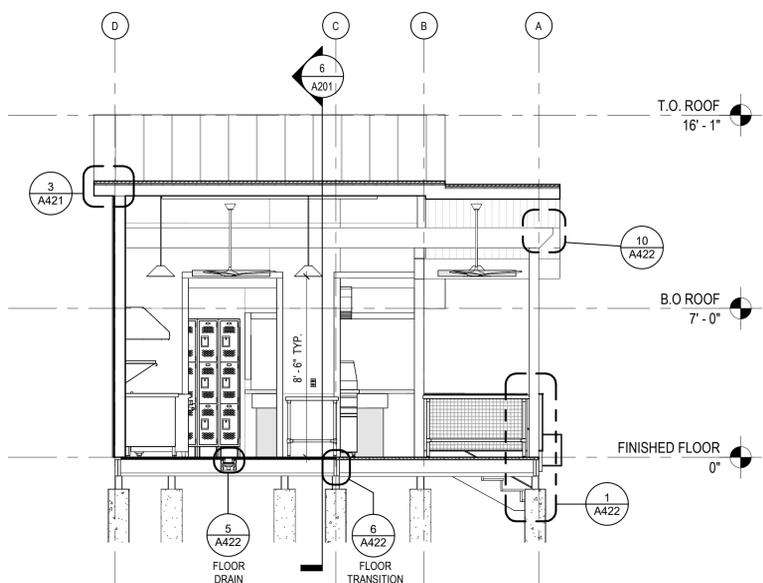
- PROVIDE BLOCKING FOR WALL MOUNTED ITEMS, EQUIPMENT AND HANDRAILS.
- CONTRACTOR TO VERIFY ELECTRICAL REQUIREMENTS FOR ALL WALL AND SURFACE MOUNTED ITEMS, EQUIPMENT AND ACCESSORIES.
- DISTANCE FROM EDGE OF DOOR FRAME TO NEAREST WALL TO BE 4", U.N.O.

GENERAL REFLECTED CEILING PLAN NOTES

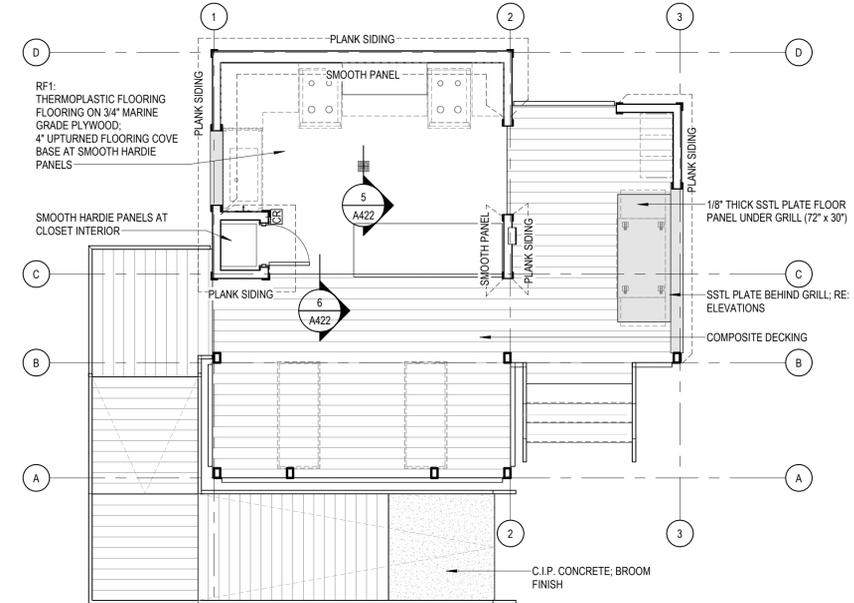
- SWITCH EACH ROOM SEPARATELY U.N.O.
- LOCATE SWITCHES PER TYPICAL DEVICE MOUNTING LEGEND.
- ALL FIRE ALARM DEVICES TO BE CEILING MOUNTED. SUBMIT SHOP DRAWINGS TO ENGINEER AND ARCHITECT FOR APPROVAL.
- SUBMIT COORDINATED RCP WITH FIRE PROTECTION DEVICES, AV DEVICES, HVAC DEVICES, LIGHTING, SECURITY DEVICES, ETC. FOR ARCHITECT'S AND ENGINEER'S REVIEW.
- ALIGN AND CENTER ALL DIFFUSERS, REGISTERS, LIGHT FIXTURES, EXIT SIGNS, ETC. AT CEILING AND ON WALL U.N.O.



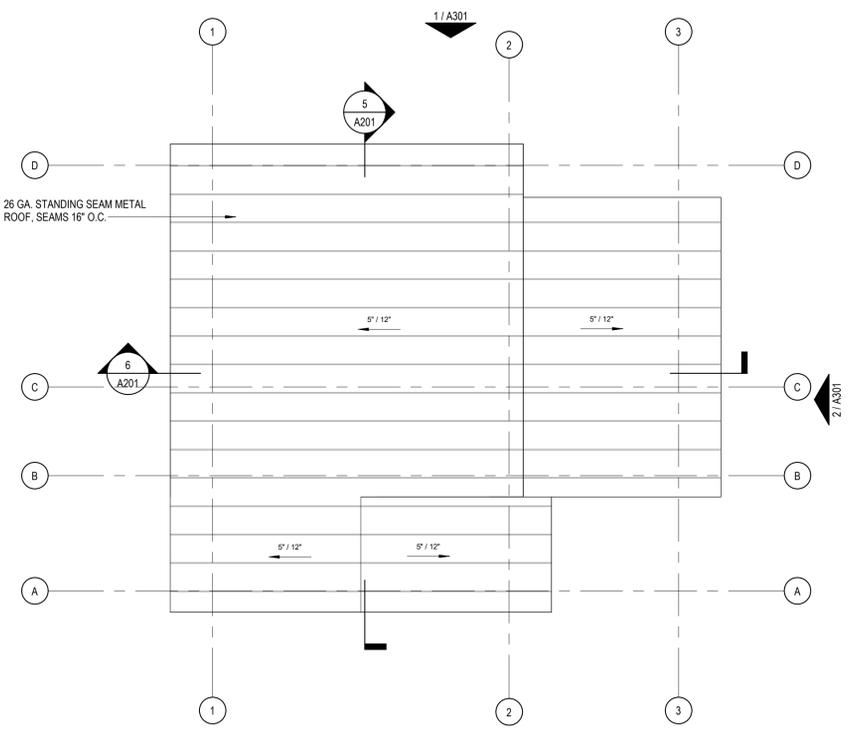
BUILDING SECTION - E/W BETWEEN A/B
1/4" = 1'-0"



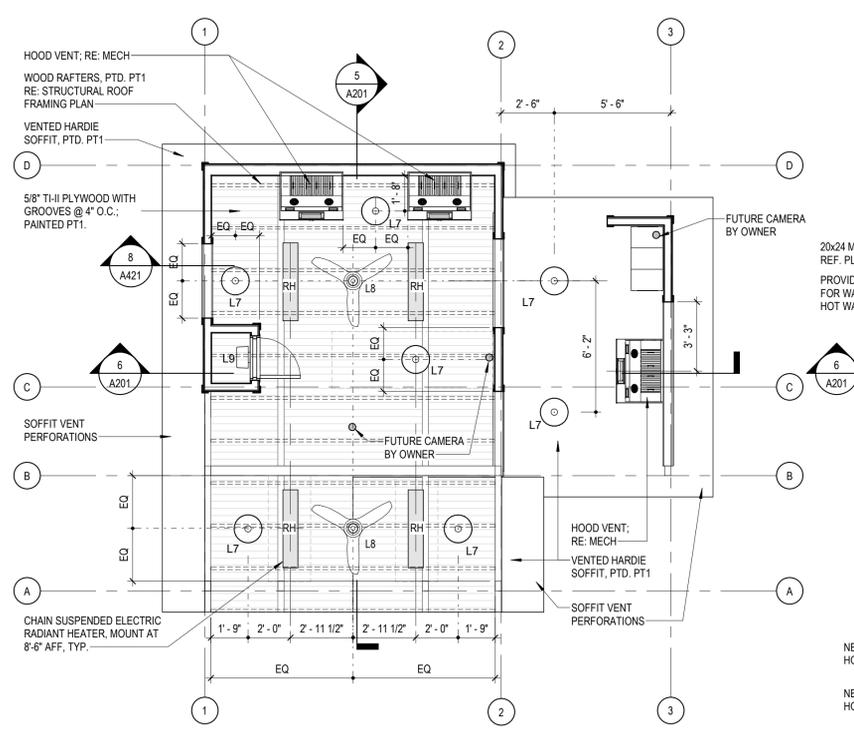
BUILDING SECTION - N/S BETWEEN 1/2
1/4" = 1'-0"



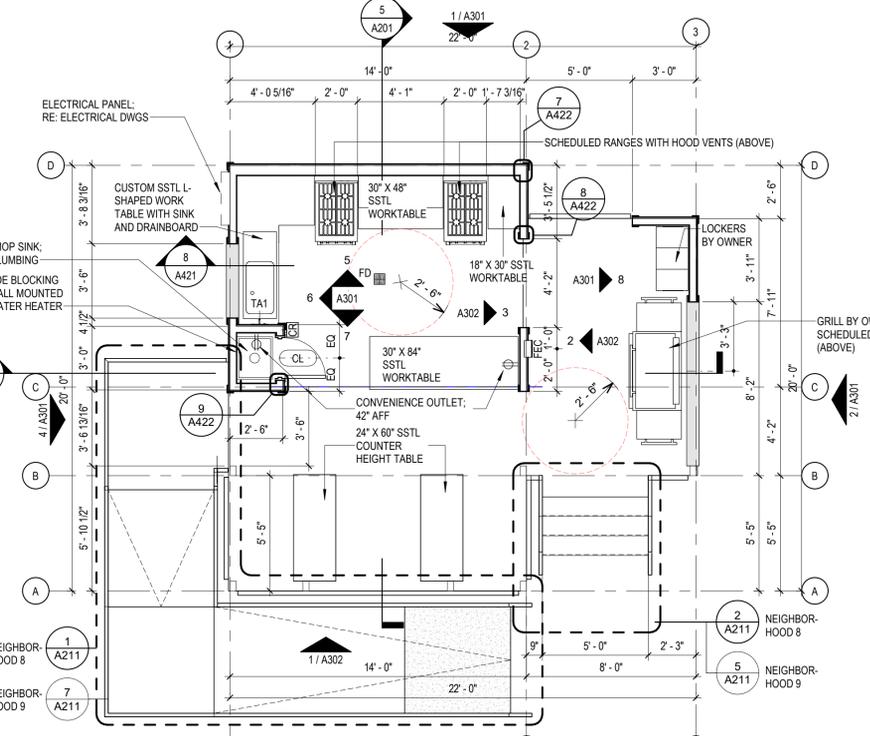
FINISH PLAN
1/4" = 1'-0"



ROOF PLAN
1/4" = 1'-0"



REFLECTED CEILING PLAN
1/4" = 1'-0"



OVERALL FLOOR PLAN
1/4" = 1'-0"

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Mobile Loaves & Fishes

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8 & 9

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PLAN NORTH



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PLANS AND SECTIONS

A201

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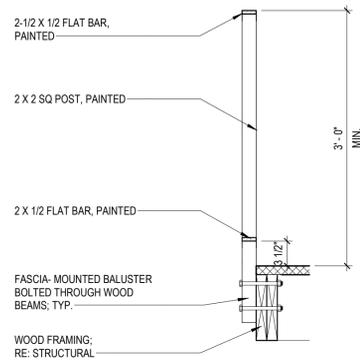
Issue
01.15.25 ISSUE FOR
CONSTRUCTION

Project Number: 24-093a

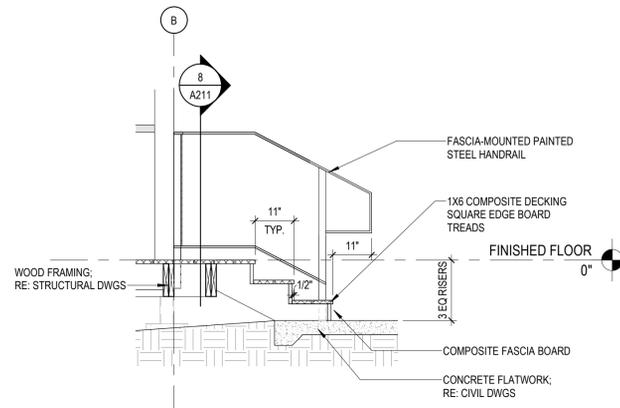
SITE RAMPS & EXTERIOR
DETAILS

A211

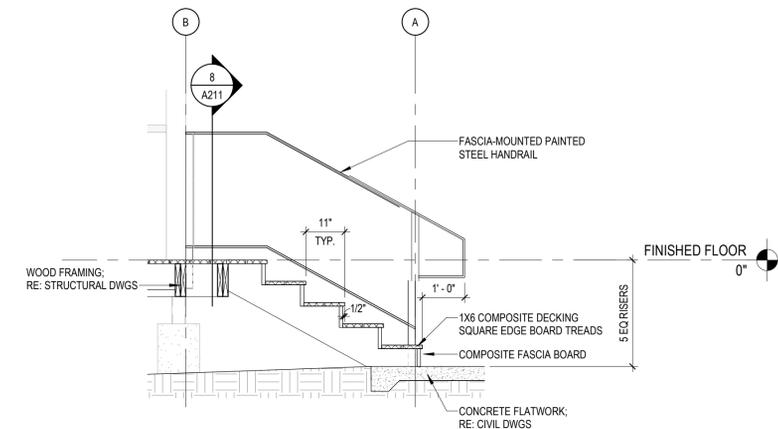
NOTES:
1. CONTRACTOR TO SUBMIT SHOP DRAWINGS OF ALL STAIR
HANDRAILS FOR ARCHITECT REVIEW PRIOR TO FABRICATION



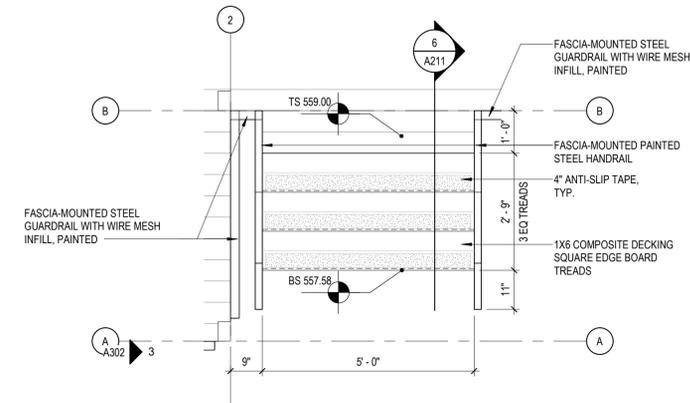
TYPICAL HANDRAIL DETAIL 8
1" = 1'-0"



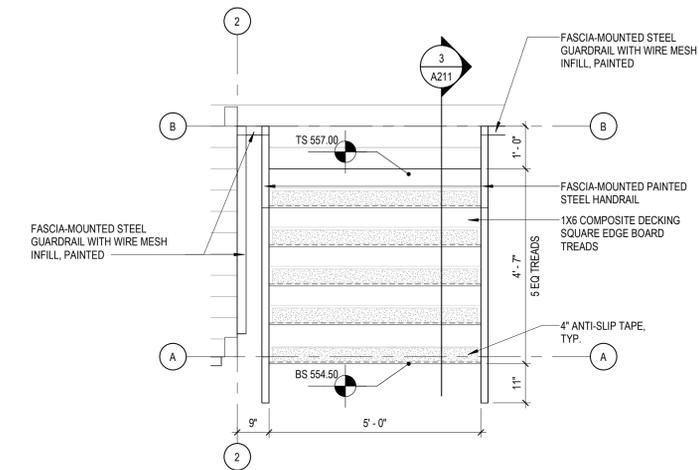
NEIGHBORHOOD 9 - KITCHEN STEPS 6
1/2" = 1'-0"



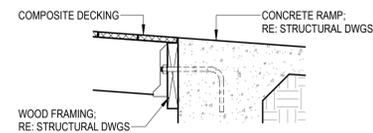
NEIGHBORHOOD 8 - KITCHEN STEPS 3
1/2" = 1'-0"



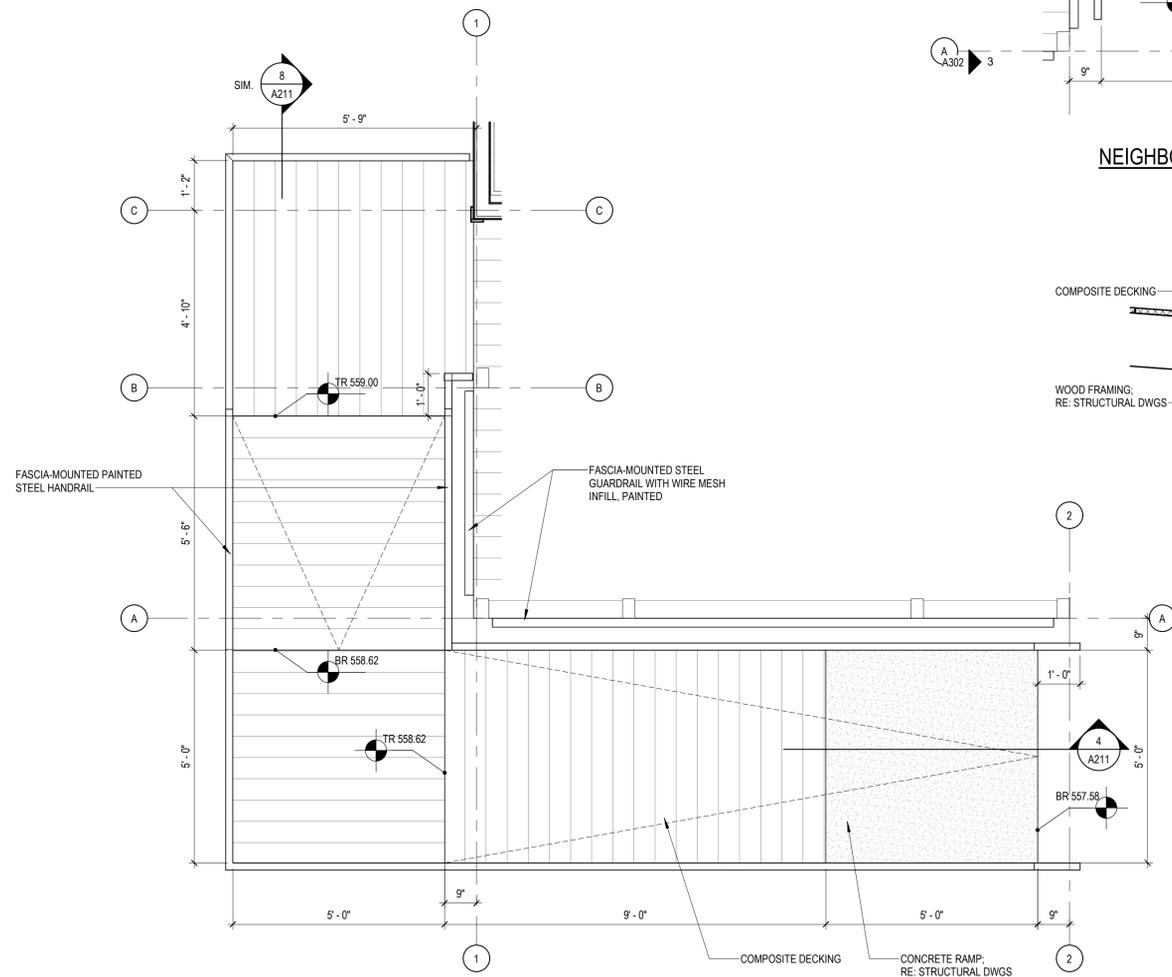
NEIGHBORHOOD 9 - KITCHEN STEPS 5
1/2" = 1'-0"



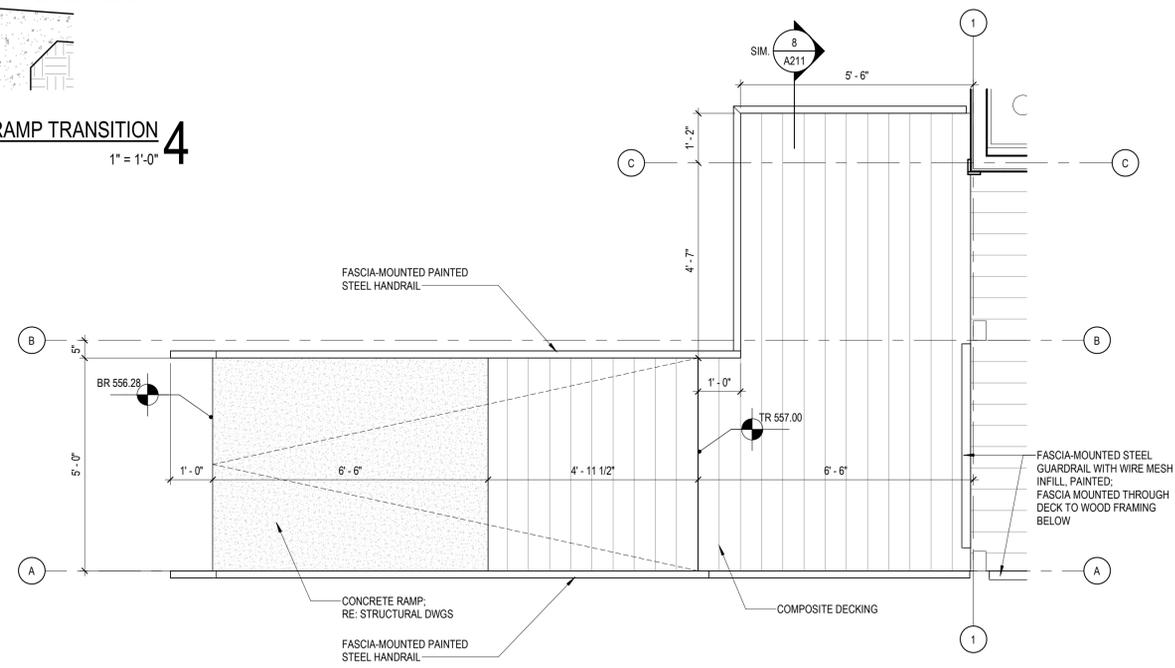
NEIGHBORHOOD 8 - KITCHEN STEPS 2
1/2" = 1'-0"



SD - TYP RAMP TRANSITION 4
1" = 1'-0"



NEIGHBORHOOD 8 - KITCHEN RAMP 7
1/2" = 1'-0"



NEIGHBORHOOD 8 - KITCHEN RAMP 1
1/2" = 1'-0"

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8 & 9**

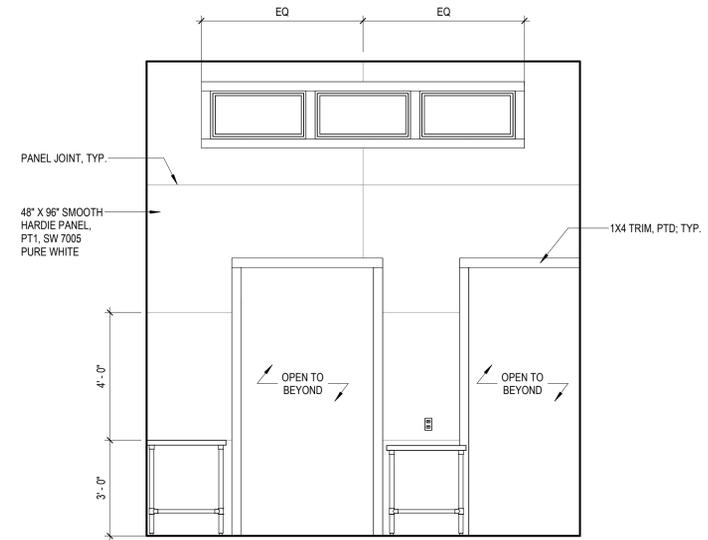
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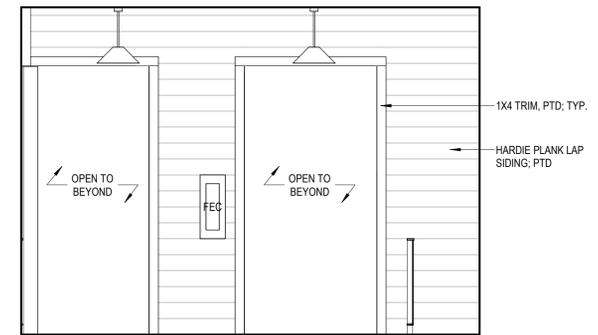
Project Number: 24-093a

ELEVATIONS

A302

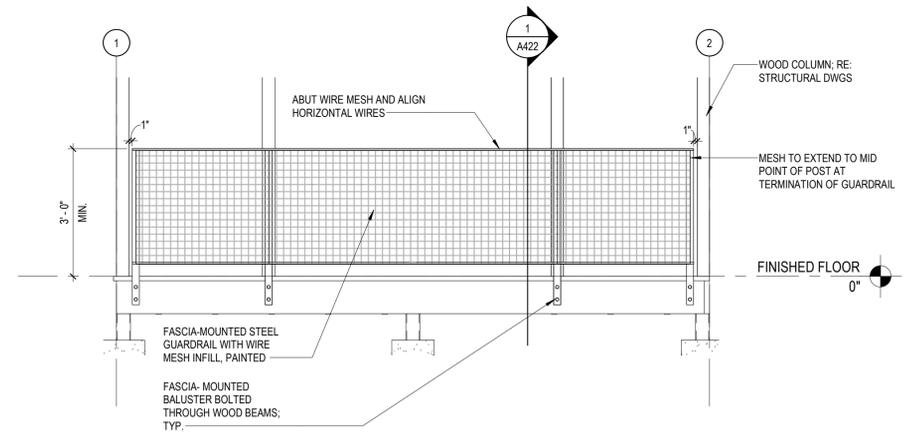


KITCHEN - PORTALS - WEST 3
3/8" = 1'-0"



KITCHEN - PORTALS - EAST 2
3/8" = 1'-0"

- NOTES:
1. CONTRACTOR TO SUBMIT SHOP DRAWINGS OF ALL GUARDRAILS FOR ARCHITECT REVIEW PRIOR TO FABRICATION
 2. TACK WELD MESH TO BARRIER FRAME AT CONSISTENT AND EVENLY SPACED LOCATIONS



TYPICAL DECK GUARDRAIL 1
1/2" = 1'-0"

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Mobile Loaves & Fishes

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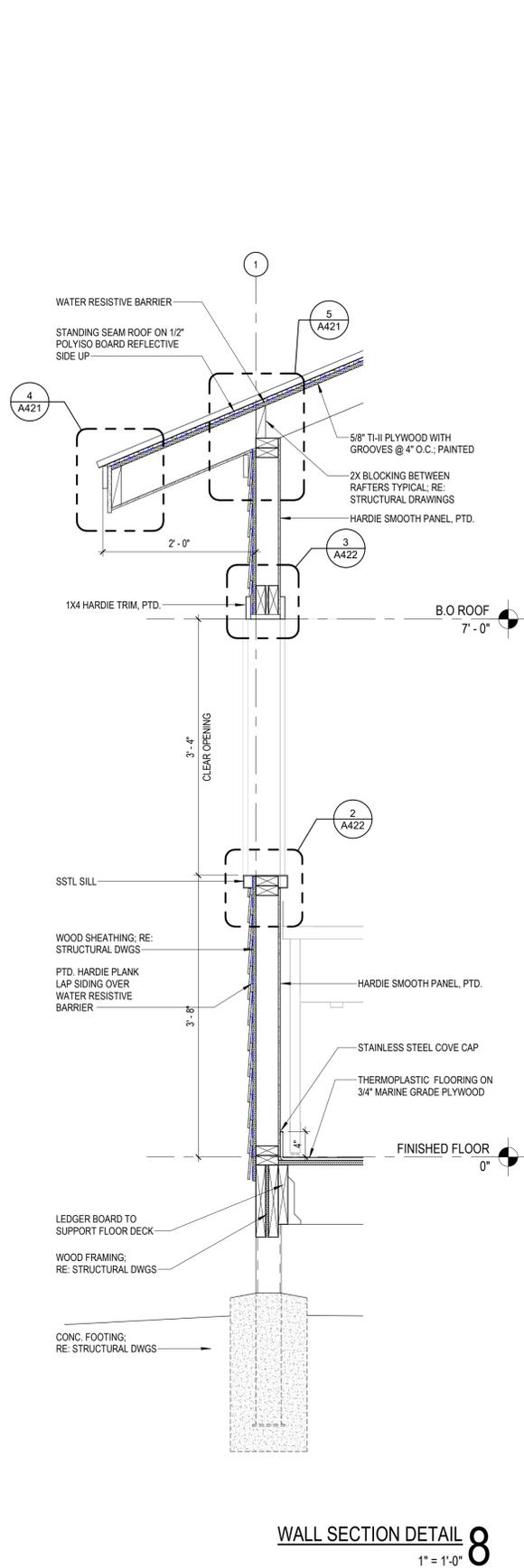
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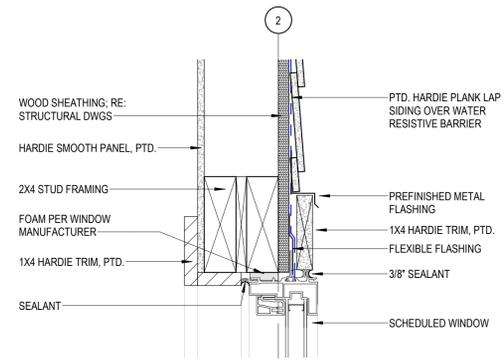
Project Number: 24-093a

SECTION DETAILS

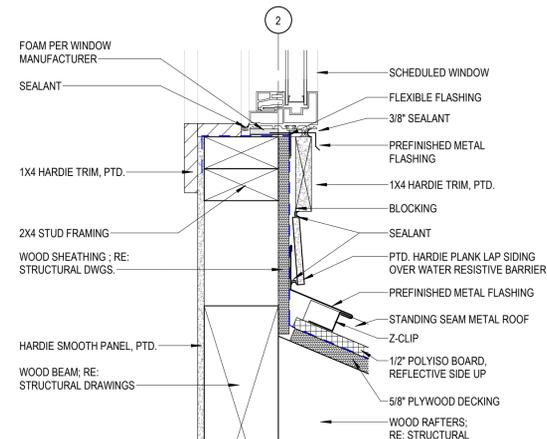
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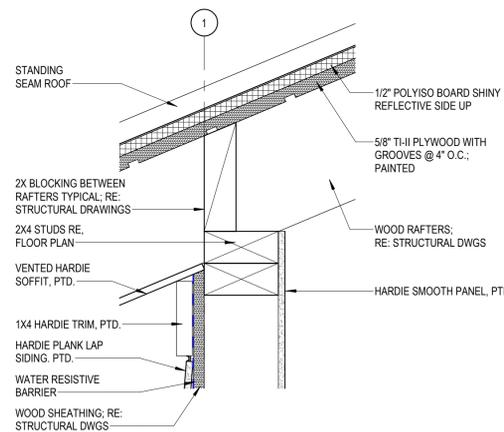
WALL SECTION DETAIL 8
1" = 1'-0"



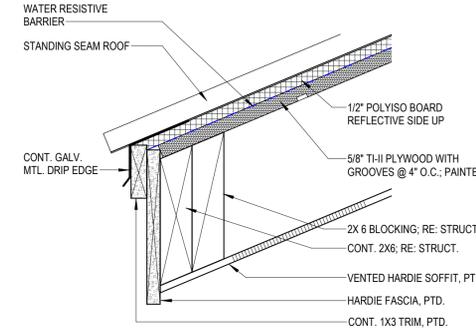
WINDOW HEAD DETAIL 7
3" = 1'-0"



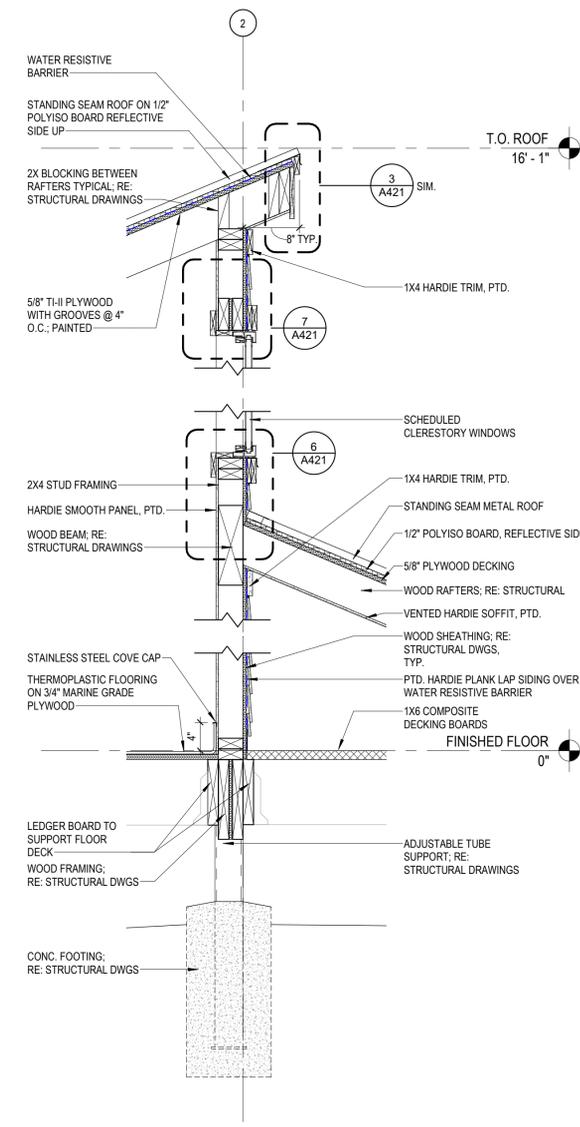
WINDOW SILL DETAIL 6
3" = 1'-0"



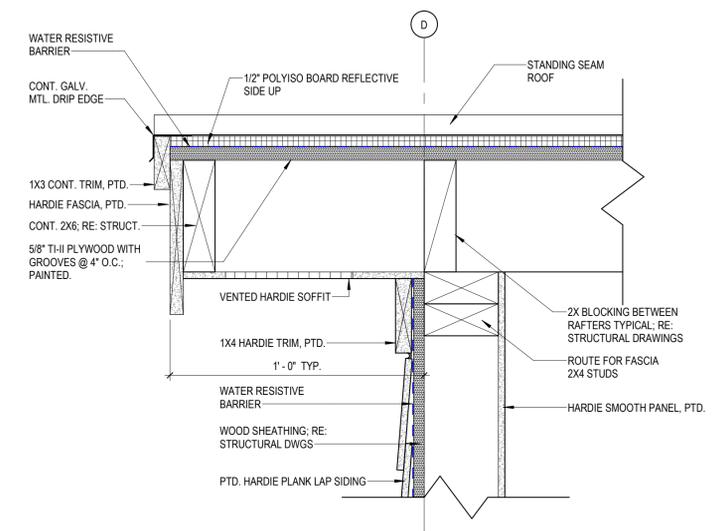
ROOF WALL CONNECTION DETAIL 5
3" = 1'-0"



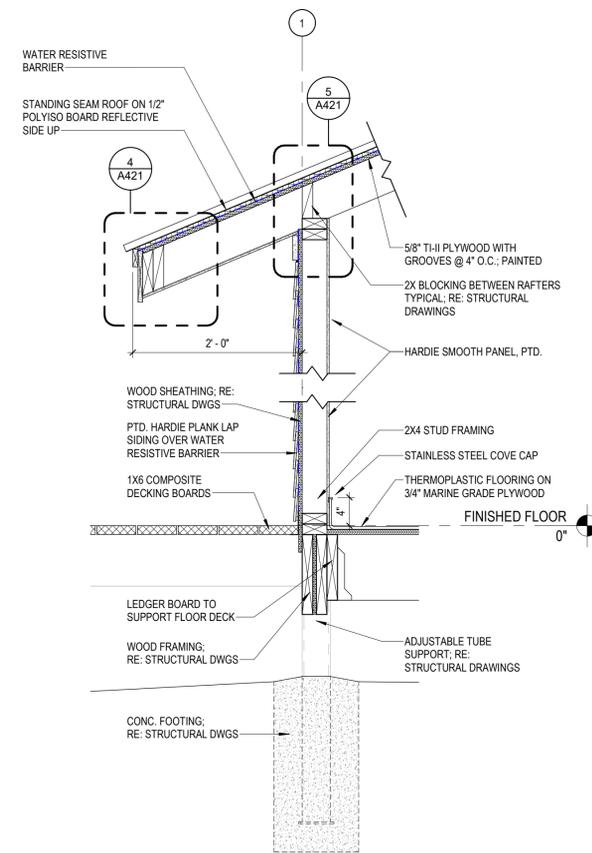
ROOF RAKE DETAIL 4
3" = 1'-0"



WALL SECTION DETAIL 2
1" = 1'-0"



ROOF OVERHANG DETAIL 3
3" = 1'-0"



WALL SECTION DETAIL 1
1" = 1'-0"

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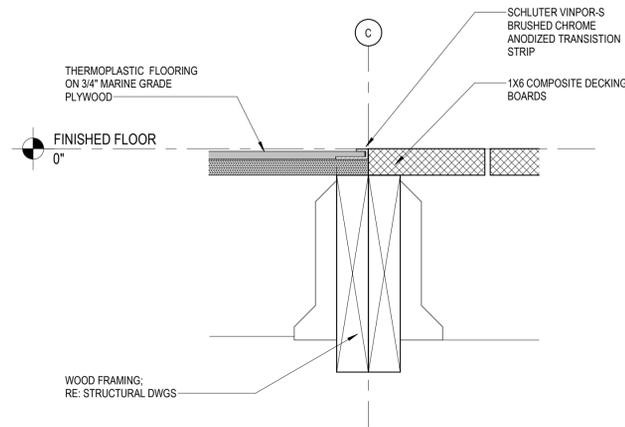
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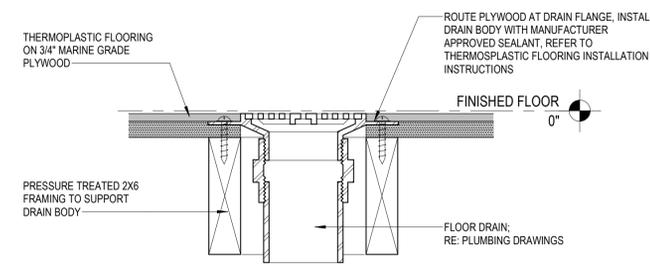
Project Number: 24-093a

PLAN AND SECTION
DETAILS

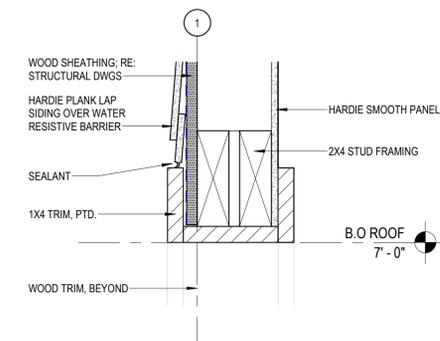
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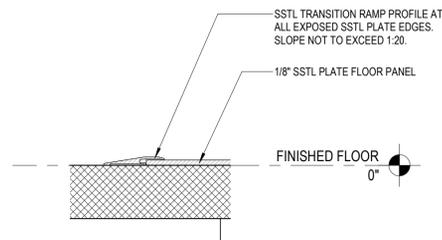
FLOORING TRANSITION DETAIL 6
3" = 1'-0"



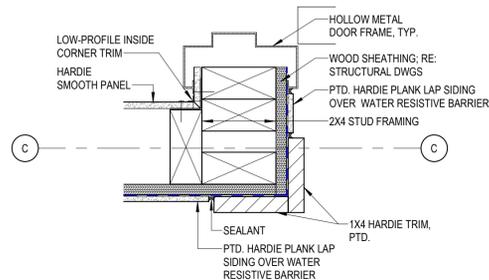
FLOOR DRAIN DETAIL 5
3" = 1'-0"



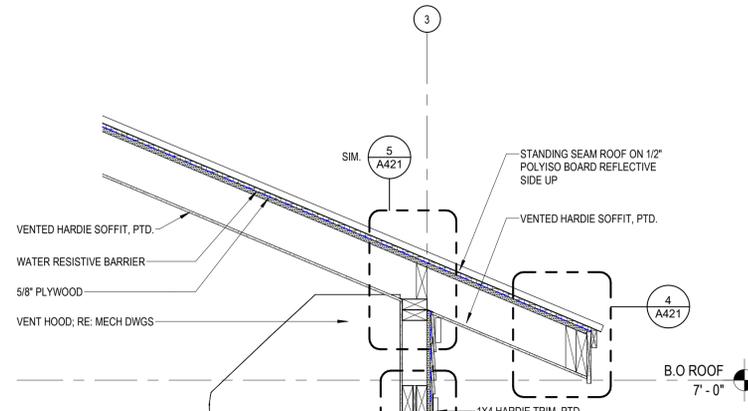
OPENING HEADER DETAIL 3
3" = 1'-0"



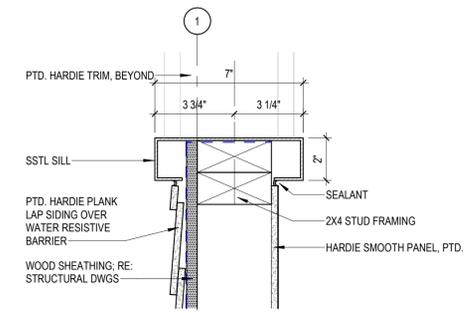
SD - SSTL PLATE TRANSITION DETAIL 11
6" = 1'-0"



04 - PD - DOOR JAMB 9
3" = 1'-0"

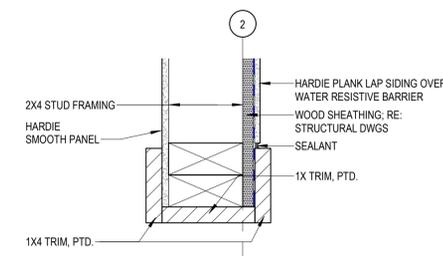


WALL SECTION DETAIL 4
1" = 1'-0"

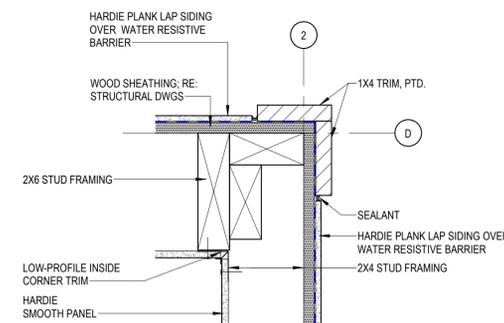


SSTL SILL DETAIL 2
3" = 1'-0"

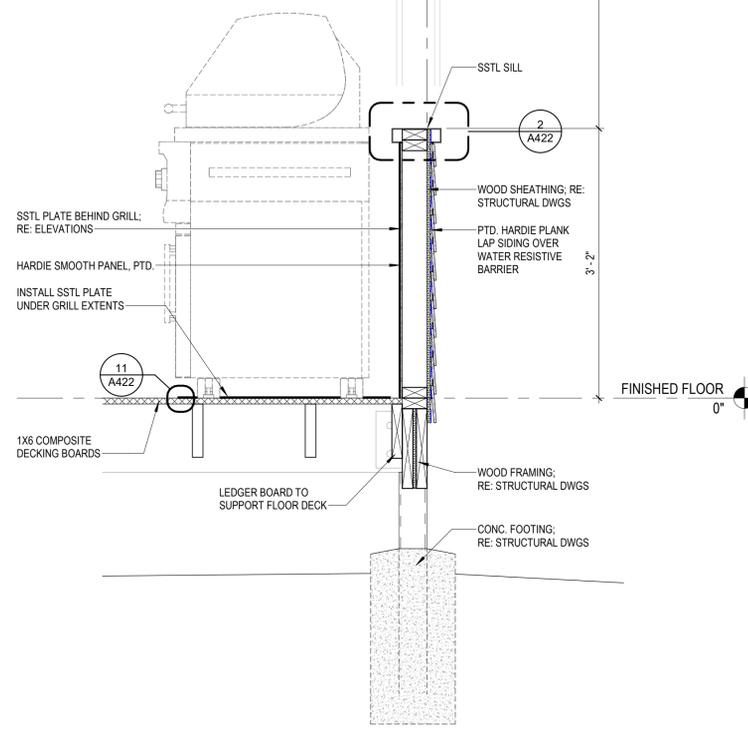
NOTES:
1. CONTRACTOR TO SUBMIT SHOP DRAWINGS OF ALL GUARDRAILS FOR ARCHITECT REVIEW PRIOR TO FABRICATION
2. TACK WELD MESH TO BARRIER FRAME AT CONSISTENT AND EVENLY SPACED LOCATIONS



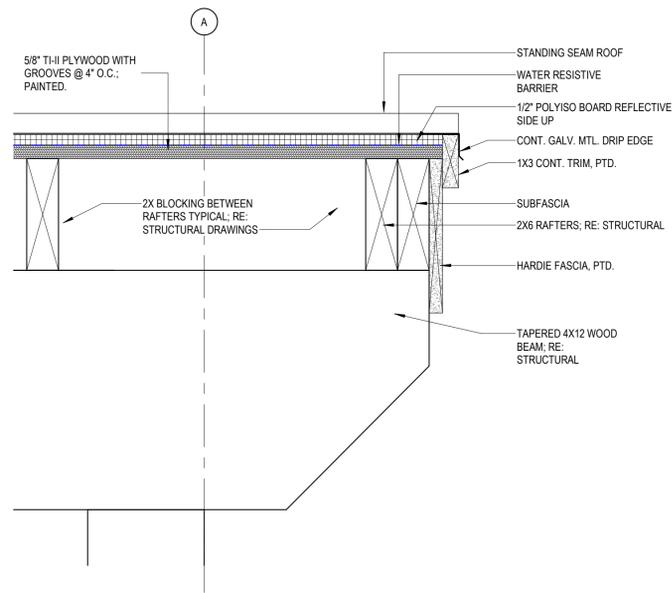
04 - PD - WALL OPENING 8
3" = 1'-0"



04 - PD - TYP CORNER 7
3" = 1'-0"



TYPICAL DECK GUARDRAIL DETAIL 1
1" = 1'-0"



SD - BEAM END 10
3" = 1'-0"

STRUCTURAL NOTES

CODES

- Building Code: International Building Code, 2021 Edition
- Minimum Design Loads: American Society of Civil Engineers, ASCE 7-16
- Structural Concrete: Building Code Requirements for Reinforced Concrete, American Concrete Institute, ACI 318-19.
- Concrete Masonry: Building Code Requirements for Masonry Structures, The Masonry Society, TMS 402-2016.
- Structural Steel: Specification for Structural Steel Buildings, Load and Resistance Factor Design, American Institute of Steel Construction, AISC 360-16.
- Wood Framing: National Design Specifications for Wood Construction with Supplement, American Wood Council, ANC NDS - 2018.
- Structural Plywood: Composite Panel Association, ANSI A195.6-2020.
- Prefabricated Metal Plate Connected Wood Trusses: Truss Plate Institute, TPI 1-2014.

SUBSTITUTIONS

- All requests for substitutions of materials or details shown in the contract documents shall be submitted for approval during the bidding period. Once bids are accepted, proposed substitutions will be considered only when they are officially submitted with an identified savings to be deducted from the contract.

DESIGN LOADS

1. Live Loads

- Dining areas and restaurants 100 psf
- Partition at areas with less than 80 psf live load 15 psf
- Roof 20 psf
- Restrooms 50 psf

x. Wind Lateral Load on Structural Frame is based on the following:

Basic Wind Speed	105 mph
Exposure	B
Risk Category	II

- Net uplift wind load -
 - Net uplift wind load on canopies -
- aa. Seismic

$I_e = 1.0$
 $S_s = .14g$
 $S_1 = .03g$
Site Class = D
 $S_{ds} = .128g$
 $S_{d1} = .064g$
Seismic Design Category A
Analysis Method
Notional Lateral Loads for SDC A
Basic Seismic Force Resisting System
Braced Frames
Design Base Shear = ,
 $C_s = .04$
 $R = 2$

bb. Ground Snow Load $P_g = 5$ psf

- Dead Loads include the self weight of the structural elements and the following superimposed loads:

- Ceiling and Mechanical at roof 10 psf
- Ceiling and Mechanical at floors 5 psf
- Roofing and rigid insulation 15 psf
- Access flooring 10 psf

- Loading for mechanical rooms and kitchens are based on the weights of equipment and concrete pads as indicated on the contract documents. Any revisions in equipment type, size, or quantity shall be reported to the Architect immediately for verification of the structural design.

BUILDING PAD PREPARATION

- Structural fill material shall meet USCS Classification CL, SC, and/or GC. Acceptable specifications include:

TxDOT Item 247, Type A, Grade 3, OR

Percent retained on No 4 sieve less than or equal to 40 percent with a plasticity index between 7 and 20, and rocks less than 4 inches, OR

Crushed concrete (TxDOT Item 247, type D, Grade 3 or better)

- Prior to placing fill material, remove all vegetation, loose fills, top soils, construction debris, and other unsuitable material organic from the existing subgrade under the building line. Remove existing material to a depth of 5.5 feet below existing grade. Where possible, proof-roll the exposed subgrade and remove weak areas detected. All exposed surfaces shall then be evaluated for moisture and density through field density testing. If the moisture and/or density test results do not meet the moisture and density requirements below, the subgrade should be scarified to a depth of 6 inches, moisture conditioned and compacted as per the fill compaction requirements.

- Structural fill shall be placed in 8 inch loose lifts, watered as required and compacted to a minimum of 95 percent of the maximum dry density as defined in ASTM D 698 at a moisture content within 3 percent of the optimum moisture content.

- Compaction and moisture content of subgrade and each lift of structural fill shall be inspected and approved by a qualified engineering technician, supervised by a Geotechnical Engineer.

- Structural fill shall not be placed beyond the limits of the exterior building structure.

- Provide a vapor barrier underneath the structural slab. Place the vapor barrier in accordance with manufacturer's recommendation on top of structural fill. The vapor barrier shall meet or exceed the following requirements:
 - Maintain permeance of less than 0.01 Perms (grains/H₂ x hr x in/Hq) as tested in accordance with mandatory conditioning tests per ASTM E 1745 Section 7.1 (7.1.1-7.1.5)
 - Strength: ASTM E 1745 Class A
 - Thickness: 15 mils minimum

- The ground surface around the building as well as the paved areas shall be sloped away from the building on all sides so that water will drain away from the structure.

- In areas beneath the slab where compacted fill depths exceed 4'-0", all utilities, exhaust lines and conduit, including but not limited to plumbing, gas, and electric conduit lines, shall be adequately attached to the underside of the concrete floor slab. Means and method of attachment shall be the responsibility of the contractor and do not fall under the scope of these structural documents.

- Building pad preparation information is based on a Geotechnical Engineering Report provided by Terracon dated May 10, 2021, with a supplemental letter dated April 18, 2024.

CAST IN PLACE CONCRETE

- Cast in place concrete shall meet the following requirements:

Class	28 Day Strength	Aggregate Type	Slump	Use
A	3000 PSI	C33	1"	3'-6" U.N.O.

- Fly ash meeting ASTM C 618 shall constitute 25% to 40% of the cementitious materials.

- Provide 5 percent plus or minus 1 1/2 percent of entrained air in concrete permanently exposed to the weather and elsewhere at the contractor's option.

- Horizontal construction joints in concrete pours shall be permitted only where indicated on the drawings. All vertical construction joints shall be made in the center of spans in accordance with the typical details. Contractor shall submit proposed locations for construction joints not shown on drawings for review by the Architect and Structural Engineer. Additional construction joints may require additional reinforcing as specified by the Engineer which shall be provided by the contractor at no additional cost to the owner.

- Embedded conduits, pipes, and sleeves shall meet the requirements of ACI 318-14, Section 26.8, including the following:

- Conduits and pipes embedded within a slab, wall, or beam (other than those passing through) shall not be larger in outside dimension than 1/3 the overall thickness of the slab, wall or beam in which they are embedded.

- Conduits, pipes and sleeves shall not be spaced closer than three diameters or widths on center.

- Provide broom finish for exterior walk surfaces, troweled finish at interior surfaces.

CONCRETE REINFORCING

- Reinforcing steel shall be deformed new billet steel bars in accordance with ASTM A615 Grade 60.

- Detailing of reinforcing steel shall conform to the American Concrete Institute Detailing Manual.

- All hooks and bends in reinforcing bars shall conform to ACI detailing standards unless shown otherwise.

- Provide reinforcing bars in accordance with the bar bending diagram if bar types are specified. In unscheduled beams, slabs, columns and walls detail reinforcing as follows:

- Lap reinforcing bars 38 bar diameters minimum, unless noted otherwise.
- Provide standard hooks in top and bottom bars at cantilever and discontinuous ends of beams, walls and slabs.
- Provide corner bars for all horizontal bars at the inside and outside faces at the terminating end of all beams or walls. Corner bars are not required if horizontal bars are hooked.

- Welding of reinforcing steel will not be permitted.

- Heat shall not be used in the fabrication or installation of reinforcement.

- Reinforcing steel minimum clear cover shall be as follows:

- Concrete formed by earth 3"
- Concrete exposed to earth or weather
No 6 bar or larger 2"
No 5 bar or smaller 1 1/2"
- Concrete not exposed to earth or weather
Slabs, Walls, or Joists 3/4"
Beams and Columns 1 1/2"

ADHESIVE DOWELS

- Adhesive dowelling system in concrete shall be one of the following products: Hilti "RE 500-V3" (ICC ESR-3814) epoxy, or Simpson Anchor Systems "SET-36" (ICC ESR-2508), Powers PE 1000+ (ICC ESR-2583), or Powers Pure 110+ (ICC ESR-3298) epoxy. Install dowels in accordance with the manufacturer's instructions. Special inspection shall be continuous and per the current ICC ES report.

- Clean out holes with compressed air after drilling holes.

Rebar Size	Hole Diameter	Min. Embedment Depth
#4	5/8"	4 1/2"
#5	3/4"	6"

- Prior to drilling holes for dowels, locate existing reinforcing steel with a Pachometer (R-Meter) or by drilling 1/4" diameter pilot holes. Relocate bolt holes as required to avoid existing reinforcement.

- Abandoned holes shall be completely filled with adhesive dowelling compound.

- Installation of adhesive anchors at an angle from horizontal to vertical (overhead) orientation shall be done by a certified adhesive anchor installer (AAI) as certified through ACI and in accordance with ACI 318-14 (section 17.8.2.2). Proof of current certification shall be submitted to the engineer for approval prior to commencement of installation.

STRUCTURAL STEEL

- Structural Steel shall conform to ASTM A992, grade 50 except where A36 is noted on plan, except that miscellaneous plates, angles, and channels may be A992, grade 50 or A36. Steel pipe shall conform to ASTM Specification A 501 or ASTM A 53, Type E or S, Grade B. Steel tube shall conform to ASTM Specification A500, Grade B, Fy 46 ksi.

- Splicing of structural steel members is prohibited without prior approval of the Engineer as to location and type of splice to be made. Any member having splice not shown and detailed on shop drawings will be rejected.

STRUCTURAL STEEL CONNECTIONS

- Welding shall conform to ANSI/AWS D1.1, latest edition.

- Bolts shall conform to ASTM A325. Bolts shall be designed using values for bearing type bolts with thread allowed in the shear plane.

- Structural steel connections not specifically detailed on the Drawings shall be designed and detailed by the Contractor under the direct supervision of a registered engineer licensed in the State of Texas. Sealed calculations for all connections designed by the Contractor shall be submitted for the Architect's files.

- Beam connections shall be designed and detailed as follows, unless noted otherwise on the Drawings:

- Connections shall be AISC type 2 simple framing connections.

- In general, shop connections shall be bolted or welded and field connections shall be welded. .

- If not indicated on the Drawings, connections shall be designed for 55 percent of the total load capacity for the beam span shown in the beam tables in the AISC LRFD Manual, 360-10.

- The minimum number of rows of bolts shall be 1/6 of the beam depth with any fraction be rounded to the next higher number.

- Bolts shall be "snug tight", u.n.o.

- Short slotted holes shall be permitted provided washers are installed in accordance with AISC requirements. Washers shall be hardened where A325 bolts are utilized.

- For connections not specifically addressed by these notes or the Drawings, provide fillet welds at all contact surfaces sufficient to develop the tensile strength of the smaller member at the joint.

- Fillet welds with no size specified shall be 3/16" or minimum size required by AISC, whichever is larger.

- Field welding of pre-galvanized structural members shall be done with care to prevent the inhalation of weld fumes. A cold galvanizing zinc rich paint shall be applied to all welds associated with galvanized steel.

TIMBER FRAMING

- Unless otherwise noted, all structural framing lumber shall be clearly marked no. 2 southern yellow pine, except that non-loadbearing interior walls may be stud grade southern yellow pine, douglas fir, or spruce-pine-fir.

- Exterior stud walls shall be 2x6's @16" on center for walls up to 12'-2". Interior load bearing stud walls shall be 2x4's @ 16" o.c. up to 12'-2" in height. Any load bearing wall taller than 12'-2" shall be 2x6 laminated strand lumber (LSL) spaced at 16" o.c. Load bearing 2x6 walls up to 12'-2" in height shall be no. 2 southern yellow pine, no. 2 douglas fir, or no. 2 spruce-pine-fir.

- All wood headers, beams, and top plates shall be no. 2 Southern Yellow Pine, U.N.O.

- Wood beams shall have a direct load path to the foundation with a minimum number of studs and blocking below each bearing point equal to the width of the supported beam.

- All wood stud walls shall be full height without intermediate plate line unless detailed otherwise.

- All load bearing walls shall have solid 2x blocking at 4'-0" o.c. maximum vertically. End nail with 2-16d nails or side toe nail with 2-16d nails.

- Provide double studs at all wall corners and on each side of all openings, unless noted or detailed otherwise.

- Floor sheathing: 1 1/8" APA rated tongue and groove sheathing with an Exposure 1 rating or 1 1/8" grade C-D tongue and groove plywood with exterior glue. Provide 1/8" joints between all sheets of plywood sheets. Stagger joints in sheathing. Floor sheathing shall be glued to the wood support members with a wet use adhesive. In addition sheathing shall also be nailed to the supports with 10d common nails at 6" on center at supported edges and 12" on center at intermediate supports.

- Roof sheathing: 15/32" APA rated sheathing with an exposure 1 rating or 15/32" grade C-D plywood with exterior glue. Panels shall be continuous over two or more spans with the long dimension oriented perpendicular to the framing members. Provide 1/8" joints between all sheets of plywood. Stagger joints in sheathing. Fasteners shall be 8d common nails. Nails shall penetrate supporting member by 1.75" and shall be spaced at 6" on center at supported edges and 6" on center at intermediate supports.

- All exterior wall framing shall be braced by 4'-0" wide x 15/32" panels of APA rated sheathing with an exposure 1 rating extending from the top plate to the sill plate. Where wall is taller than 8'-0", provide multiple panels as required to extend from sill plate to top plate. Provide 2x blocking as required to support all panel edges. Fasteners may be 8d common nails. Nails shall penetrate supporting member by 1.75". Fastener spacing shall be as specified below:
??a. Southern Yellow Pine or Douglas Fir exterior wall framing: Nails shall be spaced at 6" on center at supported edges and 12" on center at intermediate supports.

- Spruce-Pine-Fir: Nails shall be spaced at 4" on center at supported edges and 8" on center at intermediate supports.

- All interior shear walls noted on plan shall be braced by a minimum 1/2" gypsum board with No.6, 1 5/8" Type W or S screws spaced at 7" on center along the panel edges and 7" on center at interior framing members.

- Solid 2x blocking or bandboard shall be provided at supports and cantilever ends of all wood joists, and between supports in rows not exceeding 8'-0" apart.

- Provide double joists under all interior partition walls oriented parallel to the joists.

- All framing members framing into the side of a header or beam shall be attached using metal joist hangers of type "LJ" as manufactured by the Simpson Company or equal. The hanger shall be sized and installed in accordance with the manufacturers recommendations for the size of joist supported. All hangers shall be installed with 16d nails U.N.O. All pressure treated members shall be attached using stainless steel hangers.

- Nailing and attachment of all framing members and sheathing shall be as specified in the International Building Code Nailing Schedule unless noted otherwise in the drawings. Common wire nails or spikes, or galvanized box nails shall be used for all framing unless noted otherwise.

- Place a single plate at the bottom and a double plate at the top of all stud walls. Exterior sill plates shall be bolted to the foundation with 1/2" double hot dipped galvanized or stainless steel anchor bolts with a minimum embedment of 7" spaced at 4'-0" on center. Provide a minimum of two bolts per plate segment. As an alternate, attach sill plates with a triple zinc (Z-max) Simpson MAS sill plate connector @ 48" o.c. Sill plates in contact with concrete or masonry shall be pressure treated with a preservative. All interior load bearing walls shall be attached to the foundation as outlined above. As an alternate, interior load bearing wall bottom plates may be attached to concrete foundation elements with powder actuated fasteners. Provide washers at least 0.08 inches thick, and 1.1 inches square or 1.425 inches in diameter at each fastener. Fasteners shall be 3" long and shall have a minimum shank diameter of 0.145 inches. Provide two fasteners located 6 and 10 inches from the end of each sill plate piece, and then at a maximum spacing of 18 inches on center maximum. At interior non-load bearing partitions, fasteners may be spaced at 36" on center, maximum. Fasteners shall be Hilti X-DNI 72P8536 pins or equal. Submit manufacturer's information on fastener to be used prior to start of construction.

- As an alternate, plates may be attached to concrete foundation elements with powder actuated fasteners. Provide washers at least 0.08 inches thick, and 0.905 inches in diameter at each fastener. Fasteners shall be 2 7/8" long and shall have a minimum shank diameter of 0.145 inches. Provide two fasteners located 6 and 10 inches from the end of each sill plate piece, and then at a maximum spacing of 18 inches on center maximum at exterior walls and at interior party walls. At interior non-load bearing partitions, fasteners may be spaced at 36" on center, maximum. Fasteners shall be Hilti X-CP 72 P8 523 pins or equal. Submit manufacturer's information on fastener to be used prior to start of construction.

- All fasteners & connectors, including nails, attached to treated lumber shall be double hot dipped galvanized, triple zinc (Zmax), or stainless steel.

- All bolts and lag screws shall have standard washers. All anchor and expansion bolts used in wood to concrete connections in crawl-space areas shall be double hot dip galvanized, triple zinc (Zmax), or stainless steel.

- Refer to the architectural drawings for additional wood framing members. Provide additional wood framing members shown on the architectural drawings even though they may not be shown on the structural drawings.

- Wood stud walls shall be capped with a double top plate installed to provide overlapping at corners and intersections with bearing partitions. End joints in top plates shall be offset at least 24" inches. Plates shall have a width equal to the width of the studs.

- Where both top plate members are discontinuous, place a 3" x 12" x 0.036" thick steel plate. Attached with 12-8d nails on each side of cut or penetration.

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Seal:



Mobile Loaves & Fishes

Community First! Village - Kitchens - Phase 3 - Neighborhoods 8 & 9

9116 Hog Eye Rd.
Austin, TX 78724

ISSUE

01.15.25 ISSUE FOR
CONSTRUCTION

Project Number: 24-093a

STRUCTURAL NOTES

S001

STRUCTURAL NOTES

PREFABRICATED METAL PLATE CONNECTED WOOD TRUSSES

- Trusses shall be designed by the Contractor in accordance with the Truss Plate Institute "National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1-2014).
- Truss members shall be clamped in a mechanical or hydraulic jig with sufficient pressure to bring members into reasonable contact at all joints during application of connector plates.
- Provide adequate erection bracing in accordance with Truss Plate Institute publication HIB-91.
- Truss Manufacturer shall provide permanent bracing as required by the design of the trusses. Erection bracing may remain in place as permanent bracing where it does not interfere with the architectural finishes.
- All timber truss members shall be Southern Yellow Pine with a maximum moisture content of 19%. Chord members shall be no. 2 or better and web members shall be no. 3 or better.
- Connection plates shall be manufactured by a NCTCA member plate manufacturer. Plates shall be 20 gauge minimum, ASTM A446 grade A steel, with a G60 galvanized coating.
- Trusses shall be designed in accordance with the following requirements:
 - Top chords shall be designed to resist the local bending induced by the floor or roof uniform load on the top chord, including dead loads from tile or concrete flooring.
 - Limit live load deflection of floor trusses to $L/360$. Total load deflections shall be limited to $L/240$. Limit live load deflections under tile or concrete floors to $L/600$.
 - Truss members and connections shall be proportioned with a maximum allowable stress increase for duration of load as follows:

Roof Loads	25 percent
Wind Loads	33 percent
 - Trusses shall be designed for the superimposed dead and live loads as noted in the Structural Notes and as indicated on the drawings. Dead loads shall not be less than the following:

Floor	15 psf
Roof	10 psf
 - Trusses shall be designed for the superimposed wind loads in accordance with the specified building code and the specified basic wind speed, exposure, and importance factor. Increase member sizes or provide additional bridging as required to resist uplift forces.
- Connect roof trusses to all bearing walls or beam supports with a type H2.5A framing anchor as manufactured by the Simpson Company or approved equal.
- Wood beams supporting roof trusses shall be connected to the supporting studs with a minimum of two type H6 hurricane ties as manufactured by the Simpson Company or approved equal. Additional hold downs may be required upon receipt and review of approved truss shop drawings.
- Truss girders shall have a direct load path to the foundation with a minimum number of studs below each bearing point equal to the number of plies of the truss. Truss girder connections to the bearing wall and wall hold downs at truss girder locations shall be specified by a Professional Engineer registered to practice in the State of Texas. These connections shall be specified upon the Engineer's receipt of approved truss shop drawings.
- Additional blocking, studs, hold downs, or other miscellaneous framing or truss connectors may be required for trusses with exceptionally high load. Any additional items required will be specified by the engineer upon receipt of approved truss shop drawings.
- For size and location of mechanical openings see mechanical drawings.
- Truss manufacturer shall submit shop drawings and calculations for review. Shop drawings shall bear the seal of a Professional Engineer registered to practice in the State of Texas.
- Floor joists shall be proven by testing as demonstrated either by ICC and NRB acceptance or through a test program meeting ICC ESR-1153.
- Tag all connection points on web members where permanent lateral bracing is required by design.
- At roof ridges and valleys not framed with hip trusses, provide blocking between trusses as required to provide continuous support for roof sheathing.

COMPOSITE WOOD MEMBERS

- Where noted on the drawings, beams shall be "Micro-Lam" LVL or "Parallam" PSL beams as manufactured by the Weyerhaeuser Company or approved equal.
- Do not notch beams. Drill holes through webs of engineered wood members for mechanical, electrical or plumbing services in accordance with the recommendations of the engineered wood product manufacturer.
- Multiple wood beams up to three members thick shall be nailed together with three rows of 16d nails at 12" on center. Four or more multiple wood beams and any multiple wood beams utilizing beams thicker than 1 3/4" shall be bolted together with 1/2" diameter bolts top and bottom at supports and ends of the beam, then at 24" on center, staggered top and bottom for the full length of the beam, unless noted otherwise on plan. As an alternative to bolts use 1/4" diameter wood screws top and bottom at supports and ends of the beam, then at 24" on center, staggered top and bottom for the full length of the beam. Screws shall penetrate all plies of members a minimum of 1 1/2".
- At beam hanger locations provide 4 additional nails or 3 additional bolts or 1/4" screws each side of hanger for additional transfer of load to all beam plies.
- Where multiples of two 1 3/4" Micro-Lam beams are noted on the drawings, contractor may provide single 3 1/2" beams in lieu of double 1 3/4" beams.
- Connectors for double 1 3/4" beams or single 3 1/2" beams shall be Simpson "HHUS410" face mounted hangers, typical u.n.o. All hangers shall be installed with 16d nails u.n.o.

INSPECTIONS

- Contractor shall notify the Engineer a minimum of 48 hours prior to the requested date of required inspections.
- Reinspections shall be required at the discretion of the Engineer.
 - REINSPECTIONS REQUIRED DUE TO INCOMPLETE WORK OR NON-CONFORMANCE OF THE CONTRACT DOCUMENTS SHALL BE BILLED AT A RATE TWICE THAT OF THE NORMAL AGREED UPON INSPECTION RATE.
- The following items shall also be required for concrete pours:
 - Contractor shall allow a minimum of 12-24 hours from the time of inspection to time of the pour for any Engineer requested corrections.
 - Placement of the concrete reinforcing, excavations, etc., and any Engineer requested corrections shall be 100% complete before pour approval will be given.

COORDINATION

- Only larger sleeve openings and framed openings in structural framing component members are indicated on the structural drawings. However, all sleeves, inserts and openings, including frames and/or sleeves shall be provided for passage, provision and/or incorporation of the work of the contract, including but not limited to Mechanical, Electrical and Plumbing work. This work shall include the coordination of sizes, alignment, dimensions, position, locations, elevations and grades as required to serve the intended purpose. Openings not indicated on the structural drawings, but required as noted above, shall be submitted to the Engineer for review.
- Refer to Architectural, Mechanical, Electrical and Plumbing drawings for floor elevations, slopes, drains and location of depressed and elevated floor areas.
- Compatibility of the structure and provisions for building equipment supported on or from structural components shall be verified as to size, dimensions, clearances, accessibility, weights and reaction with the equipment for which the structure has been designed prior to submission of shop drawings and data for each piece of equipment and for structural components. Differences shall be noted on the submittals.
- Shop drawings shall be prepared for all structural items and submitted for review by the Engineer. Contract Drawings shall not be reproduced and used as shop drawings. All items deviating from the Contract Drawings or from previously submitted shop drawings shall be clouded.
- The details designated as "Typical Details" apply generally to the Drawings in all areas where conditions are similar to those described in the details.
- The design and provision of all temporary supports such as guys, braces, falsework, supports and anchors for safety lines, cribbing, or any other temporary elements required for the execution of the contract are not included in these drawings and shall be the responsibility of the Contractor. Temporary supports shall not result in the overstress or damage of the elements to be braced nor any elements used as brace supports.

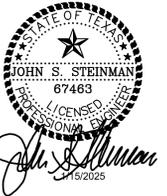
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Seal:



Mobile Loaves & Fishes

Community
First! Village -
Kitchens -
Phase 3 -
Neighborhoods
8 & 9

9116 Hog Eye Rd.
Austin, TX 78724

Issue
01.15.25 ISSUE FOR
CONSTRUCTION

Project Number: 24-093a

STRUCTURAL NOTES

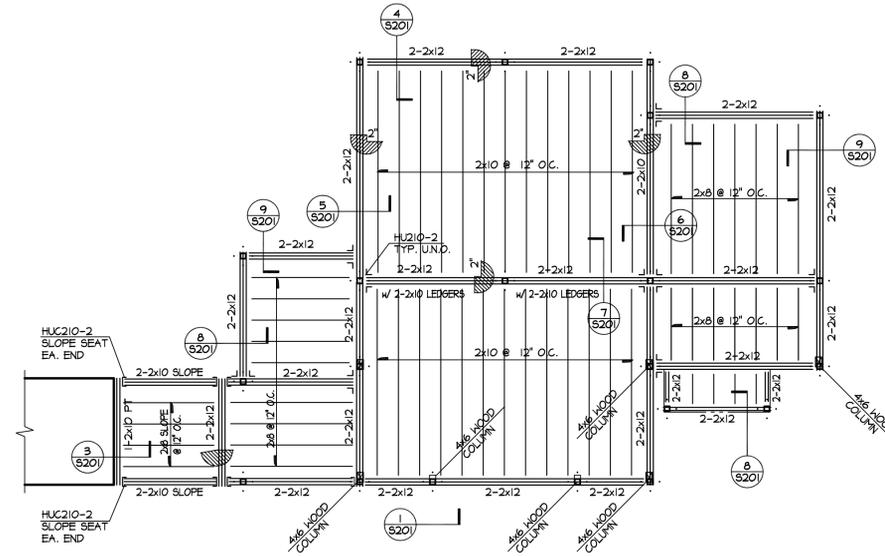
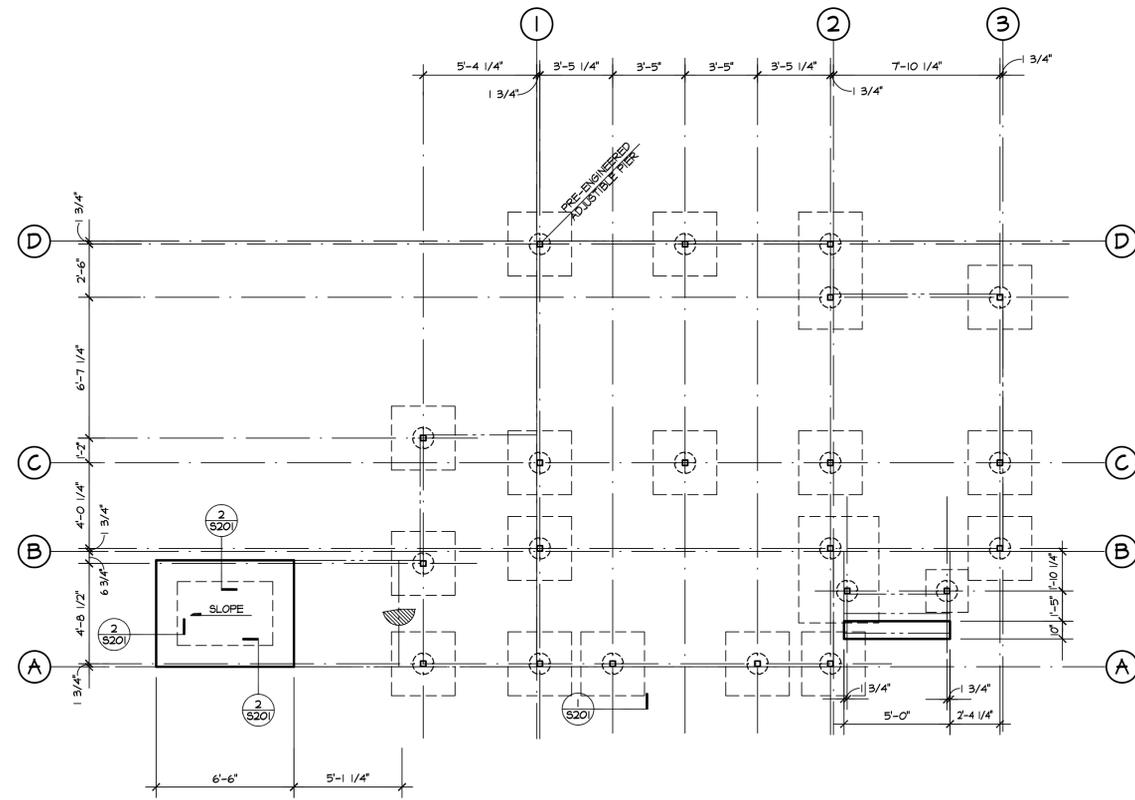
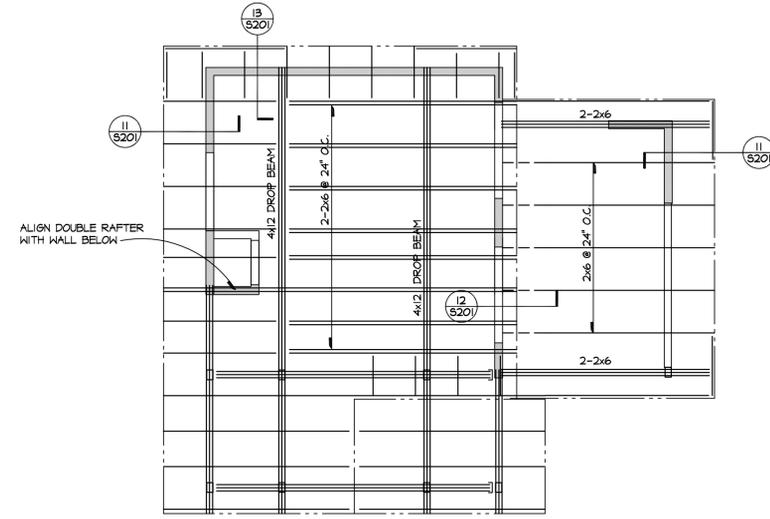
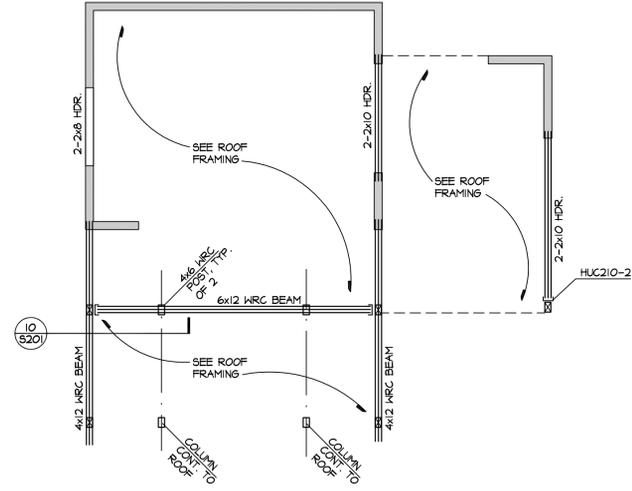
S002

STRUCTURAL TESTING AND INSPECTION REQUIREMENTS IBC 2021

REQUIRED INSPECTION VERIFICATION, OR TEST	VERIFICATION MONITORING FREQUENCY	TYPE AND/OR FREQUENCY OF TESTING	IBC SECTION & REFERENCE CRITERIA	INSPECTOR QUALIFICATIONS	REQ
1. SOILS					
			IBC 1705.6		
A. SITE PREPARATION					
1. VISUAL OBSERVATION	PERIODIC	AT THE CONTRACTOR'S EXPENSE, INSTRUMENT READINGS SHALL BE TAKEN BY A LICENSED SURVEYOR TO VERIFY FINAL SUBGRADE ELEVATIONS AND SLOPES.	GEOTECHNICAL REPORT; STRUCTURAL NOTES	QUALIFICATIONS BASED ON ASTM D3740 LICENSED SURVEYOR	✓
2. PROOFROLLING OBSERVATIONS	CONTINUOUS	PROOFROLLING SHALL BE MONITORED BY A GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER SHALL APPROVE THE TYPE OF PROOFROLLING EQUIPMENT AND PROCEDURES.	GEOTECHNICAL REPORT; STRUCTURAL NOTES	QUALIFICATIONS BASED ON ASTM D3740	
3. MOISTURE CONDITIONING & RECOMPACTION	CONTINUOUS OR PERIODIC	PROVIDE (1) ONE DENSITY TEST FOR EACH 2000 SQ. FT. REFER TO NOTES ON BUILDING PAD FOR TESTING SPECIFICATIONS.	GEOTECHNICAL REPORT; STRUCTURAL NOTES	QUALIFICATIONS BASED ON ASTM D3740	
4. BEARING	PERIODIC	VERIFY MATERIALS BELOW SHALLOW FOUNDATION ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.			
5. EXCAVATION	PERIODIC	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.			
B. CHEMICAL INJECTION	CONTINUOUS	QUALITY CONTROLLED TESTING AND EVALUATION PRIOR AND SUBSEQUENT TO INJECTION SHALL BE PERFORMED BY THE GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER SHALL APPROVE THE CHEMICAL INJECTION PROCESS. THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE SHALL MONITOR THE INJECTION PROCESS TO VERIFY AREA COVERAGE, INJECTION DEPTH AND TO REVIEW AND MONITOR THE SWELL TEST RESULTS.	GEOTECHNICAL REPORT; STRUCTURAL NOTES	QUALIFICATIONS BASED ON ASTM D3740	
C. FILL PLACEMENT	PERIODIC	PRIOR TO PLACEMENT OF COMPACTED FILL MATERIALS, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	APPROVED GEOTECHNICAL REPORT; STRUCTURAL NOTES	QUALIFICATIONS BASED ON ASTM D3740	
	CONTINUOUS	DURING PLACEMENT AND COMPACTION OF FILL, SPECIAL INSPECTOR SHALL VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS.			
D. CLAY CAP	CONTINUOUS	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS. PIT RUN MATERIALS SHALL BE VISUALLY MONITORED BY THE TESTING LAB WITH ADDITIONAL SAMPLES TESTED EACH DAY, OR MORE OFTEN IF MATERIAL APPEARS TO VARY.			
	OR PERIODIC	PLACEMENT OF CLAY CAP SHALL BE MONITORED BY GEOTECHNICAL ENGINEER WITH A WRITTEN REPORT SENT TO STRUCTURAL ENGINEER.	IBC 1704.7 GEOTECHNICAL REPORT; STRUCTURAL NOTES	QUALIFICATIONS BASED ON ASTM D3740	
2A. DRIVEN DEEP FOUNDATIONS					
			IBC 1705.7		
VERIFICATION AND INSPECTION TASK		VERIFICATION MONITORING FREQUENCY	GEOTECHNICAL REPORT; APPROVED CONSTRUCTION DOCUMENTS	GEOTECHNICAL ENGINEER	
1. VERIFY ELEMENT MATERIALS, SIZES AND LENGTHS COMPLY WITH THE REQUIREMENTS.		CONTINUOUS			
2. DETERMINE CAPACITIES OF TEST ELEMENTS AND CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED.		CONTINUOUS			
3. OBSERVE DRIVING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.		CONTINUOUS			
4. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM TYPE AND SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP AND BUTT ELEVATIONS AND DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENT.		CONTINUOUS			
5. FOR STEEL ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.2		-			
6. FOR CONCRETE ELEMENTS AND CONCRETE-FILLED ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3.		-			
7. FOR SPECIALTY ELEMENTS, PERFORM ADDITIONAL INSPECTIONS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.		-			
2B. CAST-IN-PLACE DEEP FOUNDATIONS					
			IBC 1705.8		
VERIFICATION AND INSPECTION TASK		V.M.F.	GEOTECHNICAL REPORT; CONSTRUCTION DOCUMENTS	GEOTECHNICAL ENGINEER	
1. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.		CONTINUOUS			
2. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END-BEARING STRATA CAPACITY, RECORD CONCRETE OR GROUT VOLUMES.		CONTINUOUS			
3. FOR CONCRETE ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3.		CONTINUOUS OR PERIODIC			
3. CONCRETE CONSTRUCTION					
			IBC 1705.3		
A. REINFORCING STEEL INCLUDING PRESTRESSING TENDONS AND PLACEMENT	PERIODIC	PROVIDE INSPECTION OF REINFORCING GRADE, TYPE AND SIZE; FREE OF OIL, DIRT AND RUST; LOCATED AND SPACED PROPERLY; THAT HOOKS, BENDS, TIES, STIRRUPS AND SUPPLEMENTAL REINFORCEMENT ARE PLACED CORRECTLY; LAP LENGTHS, STAGGER AND OFFSETS ARE PROVIDED AND ALL MECHANICAL CONNECTIONS ARE INSTALLED PER MANUF. INSTRUCTIONS AND/OR EVALUATION REPORT. NO FIELD WELDING PERMITTED.	ACI 318: CH. 20, 25.2, 25.3, 26.5.1-26.5.3; STRUCTURAL NOTES IBC 1908.4	QUALIFICATIONS BASED ON ASTM E329	
B. REINFORCING BAR WELDING.	PERIODIC	1. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706	AWS D1.4	AWS D1.4	
	CONTINUOUS	2. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	ACI 318: 26.5.4		
C. ANCHORS INSTALLED IN CONCRETE PRIOR TO & DURING PLACEMENT OF CONCRETE	PERIODIC	INSPECT WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	ACI 318: 17.8.2	TECHNICIAN TRAINED IN FIELD OF WORK AND HAS AT LEAST ONE YEAR OF EXPERIENCE.	
	CONTINUOUS	1. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	ACI 318: 17.8.2.4		
D. ANCHORS INSTALLED IN HARDENED CONCRETE MEMBER	PERIODIC	2. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN D.1 ABOVE	ACI 318 17.8.2		
	CONTINUOUS	EACH CONCRETE POUR	ACI 318: Ch. 19, 26.4.3, 26.4.4, IBC 1904.1, 1904.2	QUALIFICATIONS BASED ON ASTM C1077	
F. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	CONTINUOUS EACH POUR	ALL CONCRETE TESTING IS TO BE MADE AFTER WATER, IF ANY, IS ADDED AT SITE. PROVIDE A SET OF (4) FOUR CYLINDERS TO BE TAKEN FOR EVERY 75 CUBIC YARDS OF CONCRETE, OR FRACTION THEREOF, BY TESTING LAB. MONITOR SLUMP AND AIR CONTENT OF CONCRETE AND NOTIFY DELIVERY DRIVER IF SLUMP DEVIATES MORE THAN PERMITTED BY STRUCTURAL NOTES. CONTACT SUPPLIER FOR FURTHER DIRECTIONS.	ASTM C31 ASTM C172 ACI 318 - 26.5, 26.12	QUALIFICATIONS BASED ON ASTM C1077	
G. PLACEMENT OF CONCRETE & SHOTCRETE.	CONTINUOUS	INSPECT FOR PROPER APPLICATION TECHNIQUES.	ACI 318: 26.5	QUALIFICATIONS BASED ON ASTM C1077	
H. MAINTENANCE OF SPECIFIED CURING TEMPERATURE + TECHNIQUES.	PERIODIC	EACH CONCRETE POUR	ACI 318: 26.5.3-26.5.5	QUALIFICATIONS BASED ON ASTM C1077	
I. PRE-STRESSED CONCRETE.	CONTINUOUS	1. APPLICATION OF PRESTRESSING FORCE. 2. GROUTING OF BOUNDED PRESTRESSING TENDONS IN SEISMIC-FORCE RESISTING SYSTEMS.	ACI 318: 26.10	QUALIFICATIONS BASED ON ASTM C1077	
J. ERECTION OF PRECAST CONCRETE MEMBERS.	PERIODIC		ACI 318: CH.26.9	TRAINED FIELD TECHNICIAN WITH ONE YEAR MIN. EXPERIENCE.	
K. POST-TENSIONED CONCRETE:	EACH POUR	1. VERIFY IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDONS AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	ACI 318: 26.11.2		
	PERIODIC	2. THE POST-TENSIONING ENGINEER, OR A MEMBER OF HIS STAFF, SHALL INSPECT THE TENDON PLACEMENT AND CHAIRING TO INSURE COMPLIANCE WITH THE INTENT OF THE DESIGN.			
	CONTINUOUS	3. CONTINUOUS INSPECTION IS REQUIRED DURING ALL STRESSING ACTIVITIES.			
L. FORMWORK	CONTINUOUS	4. RECORDS OF ALL JACKING FORCES AND ELONGATIONS SHALL BE MADE IN ACCORDANCE WITH THE PTI FIELD MANUAL AND RECORDS SHALL BE PROMPTLY SUBMITTED TO THE ARCHITECT AND ENGINEER.		QUALIFICATIONS BASED ON ASTM E329	
	PERIODIC	INSPECT FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	ACI 318: 26.11.2(b)	QUALIFICATIONS BASED ON ASTM E329	

REQUIRED INSPECTION VERIFICATION, OR TEST	VERIFICATION MONITORING FREQUENCY	TYPE AND/OR FREQUENCY OF TESTING	IBC SECTION & REFERENCE CRITERIA	INSPECTOR QUALIFICATIONS	REQ
4. STEEL CONSTRUCTION					
			IBC 1705.2		
A. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO ATM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	STRUCTURAL NOTES	TRAINED FIELD TECHNICIAN WITH ONE YEAR MIN. EXPERIENCE	
	PERIODIC	2. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	APPLICABLE ATM MATERIAL SPECIFICATIONS; BASIC 335, SECTION A3.4; BASIC RFD, SECTION A3.3		
B. HIGH-STRENGTH BOLTING	PERIODIC	1. BEARING-TYPE CONNECTIONS.	STRUCTURAL NOTES	TRAINED FIELD TECHNICIAN WITH ONE YEAR MIN. EXPERIENCE	
	CONTINUOUS OR PERIODIC	2. SLIP-CRITICAL CONNECTIONS.	BASIC 360 NS.6 NS.6		
C. MATERIAL VERIFICATION OF STRUCTURAL STEEL	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO ATM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENT	IBC 1705.2.2; STRUCTURAL NOTES	TRAINED FIELD TECHNICIAN WITH ONE YEAR MIN. EXPERIENCE	
	PERIODIC	2. MANUFACTURERS' CERTIFIED MILL TEST REPORTS.	ATM A 6 OR ATM A 568		
D. MATERIAL VERIFICATION OF WELD FILLER MATERIALS	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO AW SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	STRUCTURAL NOTES	WI	
	PERIODIC	2. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	BASIC, SD, SECTION A3.6; BASIC RFD, SECTION A3.5		
	CONTINUOUS	1. COMPLETE & PARTIAL PENETRATION GROOVE WELDS.	STRUCTURAL NOTES	WI	
E. WELDING OF STRUCTURAL STEEL	CONTINUOUS	2. MULTITASKS FILLET WELDS.			
	CONTINUOUS	3. SINGLE-PASS FILLET WELDS > 5/16" PLUG AND SLOT WELDS	AW D1.1	WI	
	PERIODIC	4. SINGLE PASS FILLET WELDS ≤ 5/16"			
F. STEEL FRAME JOINT DETAILS: COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	1. DETAILS SUCH AS BRACING & STIFFENING.		TRAINED FIELD TECHNICIAN WITH ONE YEAR MIN. EXPERIENCE	
	PERIODIC	2. MEMBER LOCATIONS.			
	PERIODIC	3. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.			
G. INSTALLATION OF OPEN-WEB STEEL JOISTS AND JOIST ORDERS	PERIODIC	1. END CONNECTIONS - WELDING OR BOLTED	SJI SPECIFICATION LISTED IN SECTION 2207.1		
	PERIODIC	2. HORIZONTAL OR DIAGONAL STANDARD BRIDGING.			
	PERIODIC	3. ORIZ. OR DIAG. BRIDGING THAT DIFFERS FROM THE SJI SPECS LISTED IN 2207.1			
4A. STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL					
			IBC 1705.2		
A. MATERIAL VERIFICATION OF COLD-FORMED METAL DECK	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO ATM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	APPLICABLE ASTM MATERIAL STANDARDS		
	PERIODIC	2. MANUFACTURER'S CERTIFIED TEST REPORTS			
B. INSPECTION OF WELDING OF COLD-FORMED STEEL DECK	PERIODIC	1. FLOOR AND ROOF DECK WELDS	AWS D1.3		
C. INSPECTION OF WELDING OF REINFORCING STEEL	PERIODIC	1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706			
	CONTINUOUS	2. REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	AWS D1.4 ACI 318: SECTION 3.5.2		
	CONTINUOUS	3. SHEAR REINFORCEMENT			
	PERIODIC	4. OTHER REINFORCING STEEL			
5. INSPECTION OF FABRICATORS OF STRUCTURAL ELEMENTS					
			IBC 1704.2.5		
FABRICATION & IMPLEMENTATION PROCEDURES	PERIODIC	THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK. EXCEPTION: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR THAT IS ENROLLED IN A NATIONALLY ACCEPTED INSPECTIONS PROGRAM ACCEPTABLE TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO BUILDING OFFICIAL UPON REQUEST AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.	IBC 1704.2.5.1	CWI, LICENSED ENGINEER	
6. WOOD CONSTRUCTION					
			IBC 1705.5		
A. PREFABRICATED WOOD STRUCTURAL ELEMENTS & SITE BUILT ASSEMBLIES	PERIODIC	INSPECT STRUCTURAL LOAD BEARING MEMBERS AND ASSEMBLIES. VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATORS ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATORS SCOPE OF WORK. EXCEPTION: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED WHERE THE FABRICATOR IS ENROLLED IN A NATIONALLY ACCEPTED INSPECTIONS PROGRAM ACCEPTABLE TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.	IBC 1704.2.5	TECHNICAL REPRESENTATIVE UNDER DIRECTION OF LICENSED ENGINEER	
B. HIGH LOAD DIAPHRAGMS	PERIODIC	DIAPHRAGMS SHALL BE INSPECTED IN ACCORDANCE WITH IBC SECTION 1704.2, AND SHEATHINGS CHECKED FOR PROPER GRADE, THICKNESS, SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, NAIL/STAPLE DIAMETER AND LENGTH, FASTENER PATTERN, AND CONTINUITY OF LOAD PATH TO FOUNDATION.	IBC 1705.5.1	LICENSED ENGINEER OR HIS/HER REPRESENTATIVE.	
C. BRACING OF TRUSSES SPANNING ≥ 60'	PERIODIC	CHECK THAT ALL REQUIRED TEMPORARY + PERMANENT LATERAL RESTRAINT/ BRACING HAS BEEN INSTALLED ACCORDING TO STRUCTURAL DRAWINGS APPROVED TRUSS SUBMITTAL PACKAGE.	IBC 1705.5.2		

REQUIRED INSPECTION VERIFICATION, OR TEST	VERIFICATION MONITORING FREQUENCY	TYPE AND/OR FREQUENCY OF TESTING	IBC/ACI SECTION & REFERENCE CRITERIA	INSPECTOR QUALIFICATIONS	REQ
7. WIND RESISTANCE EXP CAT. B V_{wdg} ≥ 120 MPH, EXP C, D V_{wdg} ≥ 110 MPH					
			IBC 1705.11, 1704.6.2		
A. STRUCTURAL WOOD	CONTINUOUS	INSPECTION DURING FIELD GLUING OPERATIONS OF ELEMENTS OF THE MAIN WINDFORCE - RESISTING SYSTEM.			
	PERIODIC	INSPECTION FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE MAIN WINDFORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLD-DOWNS.	IBC 1705.11.1		
B. COLD-FORMED STEEL LIGHT FRAME	PERIODIC	WELDING OPERATIONS OF ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM.			
	PERIODIC	SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING COMPONENTS WITHIN THE MAIN WINDFORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, BRACES DIAPHRAGMS, COLLECTORS (DRAG STRUTS) AND HOLD-DOWNS.	IBC 1705.11.2		
C. WIND RESISTING COMPONENTS	PERIODIC	ROOF COVER, ROOF DECK AND ROOF FRAMING CONNECTIONS EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING	IBC 1705.11.3		
8. MASONRY CONSTRUCTION					
			IBC 1705.4		
LEVEL A INSPECTION		EMPIRICALLY DESIGNED MASONRY, GLASS UNIT MASONRY, AND MASONRY VENEER IN RISK CATEGORY I, II, OR III			
	SPECIAL INSPECTION NOT REQUIRED	PRIOR TO CONSTRUCTION, VERIFY COMPLIANCE WITH APPROVED SUBMITTALS	ACI TABLE 3.1.1		QUALIFICATIONS BASED ON ASTM C1093
LEVEL B INSPECTION		EMPIRICALLY DESIGNED MASONRY, GLASS UNIT MASONRY, AND MASONRY VENEER IN RISK CATEGORY IV			
		MINIMUM TESTING	ACI REFERENCE		
			ACI 530 ACI 530.1		
AT TIME OF DELIVERY TO SITE	PERIODIC	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) FOR SELF CONSOLIDATING GROUT	ART 1.5 B.1.b.3		QUALIFICATIONS BASED ON ASTM C1093
PRIOR TO CONSTRUCTION	PERIODIC	VERIFICATION OF f' _m AND f' _{acc} EXCEPT WHERE SPECIFICALLY EXEMPTED	ART 1.4 B		
		MINIMUM SPECIAL INSPECTIONS			
VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS	PERIODIC		ART 2.1, 2.6A		
AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:	PERIODIC	1. CONSTRUCTION OF MORTAR JOINTS	ART 3.3B		
	PERIODIC	2. GRADE AND SIZE OF PRE-STRESSING TENDONS AND ANCHORAGES	ART 2.4B, 2.4H		
	PERIODIC	3. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRE-STRESSING TENDONS AND ANCHORAGES	ART 3.4, 3.6A		
	PERIODIC	4. PRE-STRESSING TECHNIQUE	ART 3.6B		
	CONTINUOUS	5. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	ART 2.1C		
	PERIODIC	1. GROUT SPACE	ART 3.2D, 3.2F		
	PERIODIC	2. GRADE, TYPE, AND SIZE OF REINF., ANCHOR BOLTS, PRE-STRESSING TENDONS AND ANCHORS	SEC. 6.1	ART 2.4, 3.4	
PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:	PERIODIC	3. PLACEMENT OF REINF., CONNECTORS, AND PRE-STRESSING TENDONS AND ANCHORS	SEC. 6.1, 6.21, 6.26, 6.27	ART 3.2E, 3.4, 3.6A	
	PERIODIC	4. PROPORTIONS OF SITE-PREPARED GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS	ART 2.6B, 2.46.1.b		
	PERIODIC	5. CONSTRUCTION OF MORTAR JOINTS	ART 3.3B		
	PERIODIC	1. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	ART 3.3F		
VERIFY DURING CONSTRUCTION	PERIODIC	2. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	SEC. 12.1(a), 6.143, 6.21		
	CONTINUOUS	3. WELDING OF REINFORCEMENT	SEC. 7.2, 9.3.34 (g), 11.3.34(b)		
	PERIODIC	4. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMP BELOW 40°F) OR HOT WEATHER (TEMP ABOVE 90°F)	ART 1.8C, 1.8D		
	CONTINUOUS	5. APPLICATION AND MEASUREMENT OF PRE-SETTING FORCE	ART 3.6B		
	CONTINUOUS	6. PLACEMENT OF GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	ART 3.5, 3.6C		
	CONTINUOUS	7. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	ART 3.3B.9, 3.3F.1.b		
	PERIODIC	8. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	ART 1.4B, 2.a.3, 1.4B.2.b.3, 1.4B.2.c.3, 1.4B.3, 1.4B.4		
LEVEL C INSPECTION					
			ENGINEERED MASONRY IN RISK CATEGORY IV		
		MINIMUM TESTING	ACI REFERENCE		
			ACI 530 ACI 530.1		
AT TIME OF DELIVERY TO SITE	PERIODIC	VERIFICATION OF PROPORTIONS OF MATERIAL IN PREMIXED OR PRE-BLENDED MORTAR, PRE-STRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT			TESTING AGENCY TO COMPLY WITH ASTM C1093
AT TIME OF DELIVERY TO SITE	PERIODIC	VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) FOR SELF CONSOLIDATING GROUT	ART 1.5 B.1.b.3		
PRIOR TO CONSTRUCTION AND EVERY 5000SQFT DURING CONSTRUCTION	PERIODIC	VERIFICATION OF f' _m AND f' _{acc} EXCEPT WHERE SPECIFICALLY EXEMPTED	ART 1.4 B		
		MINIMUM SPECIAL INSPECTIONS			
1. VERIFY COMPLIANCE WITH THE APPROVED SUBMITTALS	PERIODIC		ART 1.5		
2. VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:	PERIODIC	1. PROPORTIONS OF SITE-MIXED MORTAR, GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS	ART 2.12.2(a), 2.62.2(a), 1.4B.3		
	PERIODIC	2. GRADE, TYPE, AND SIZE OF REINF., ANCHOR BOLTS, PRE-STRESSING TENDONS AND ANCHORS	SEC. 6.1	ART 2.4, 3.4	
PERIODIC	PERIODIC	3. PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS	SEC. 6.1	ART 3.3B	
	PERIODIC	4. PLACEMENT OF REINF., CONNECTORS, AND PRE-STRESSING TENDONS AND ANCHORS	SEC. 6.1, 6.21, 6.26, 6.27	ART 3.2E, 3.4, 3.6A	
CONTINUOUS	5. GROUT SPACE PRIOR TO GROUTING	ART 3.2D, 3.2F			
CONTINUOUS	6. PLACEMENT OF GROUT AND PRE-STRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	ART 3.5, 3.6C			
CONTINUOUS	7. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	ART 3.3F			
PERIODIC	8. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	SEC. 12.1(a), 6.143, 6.21			
CONTINUOUS	9. WELDING OF REINFORCEMENT	SEC. 7.2, 9.3.34 (g), 11.3.34(b)			
CONTINUOUS	10. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMP BELOW 40°F) OR HOT WEATHER (TEMP ABOVE 90°F)	ART 1.8C, 1.8D			
CONTINUOUS	11. APPLICATION AND MEASUREMENT OF PRE-SETTING FORCE	ART 3.6B			
CONTINUOUS	12. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	ART 3.3B.9, 3.3F.1.b			
CONTINUOUS	13. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	ART 2.1 C.1			
3. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	CONTINUOUS		ART 1.4B, 2.a.3, 1.4B.2.b.3, 1.4B.2.c.3, 1.4B.3, 1.4B.4		



Seal:



Mobile Loaves & Fishes

Community
First! Village -
Kitchens -
Phase 3 -
Neighborhoods
8 & 9

9116 Hog Eye Rd.
Austin, TX 78724

PLAN NORTH

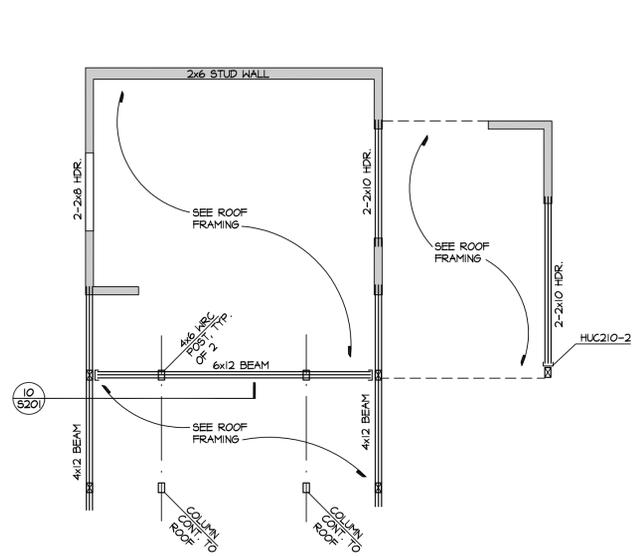


Issue
01.15.25 ISSUE FOR
CONSTRUCTION

Project Number: 24-093a

FOUNDATION AND FRAMING
PLANS

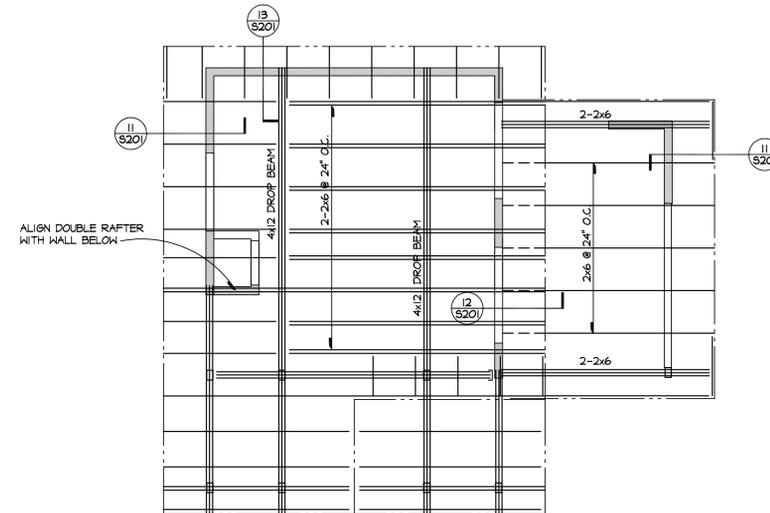
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CEILING FRAMING PLAN

SCALE: 1/4"=1'-0"

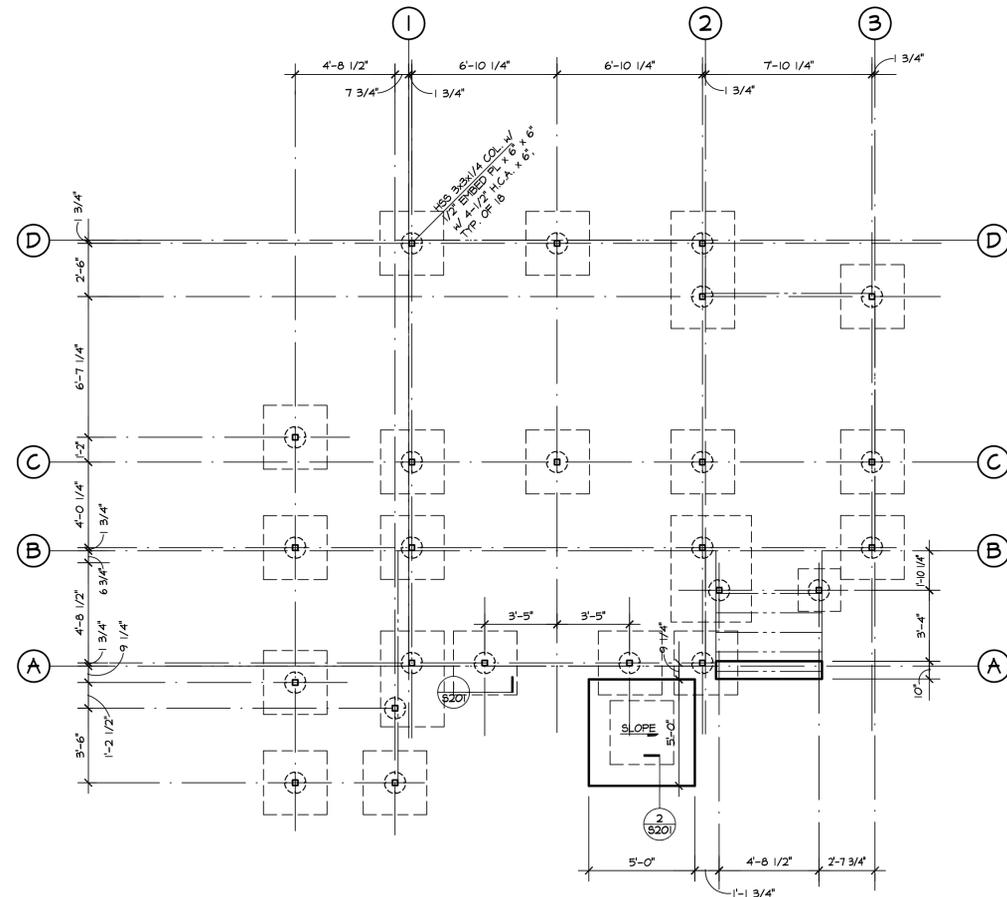
- NOTES:
1. ALL CEILING JOISTS SHALL BE 2x6 @ 24" O.C. UNLESS NOTED OTHERWISE
 2. LOAD BEARING WALLS INDICATED AS [Symbol] SHALL BE 2x STUDS @ 16" O.C.
 3. PROVIDE MINIMUM 3-2x STUDS BELOW ALL WOOD BEAMS. PROVIDE 2-2x CRIPPLES BELOW ALL HEADERS LARGER THAN 2-2x10.
 4. HEADERS IN 2x4 LOAD BEARING WALLS SHALL BE 2-2x8 UNLESS NOTED OTHERWISE. HEADERS IN 2x6 LOAD BEARING WALLS SHALL BE 3-2x8 UNLESS NOTED OTHERWISE



ROOF FRAMING PLAN

SCALE: 1/4"=1'-0"

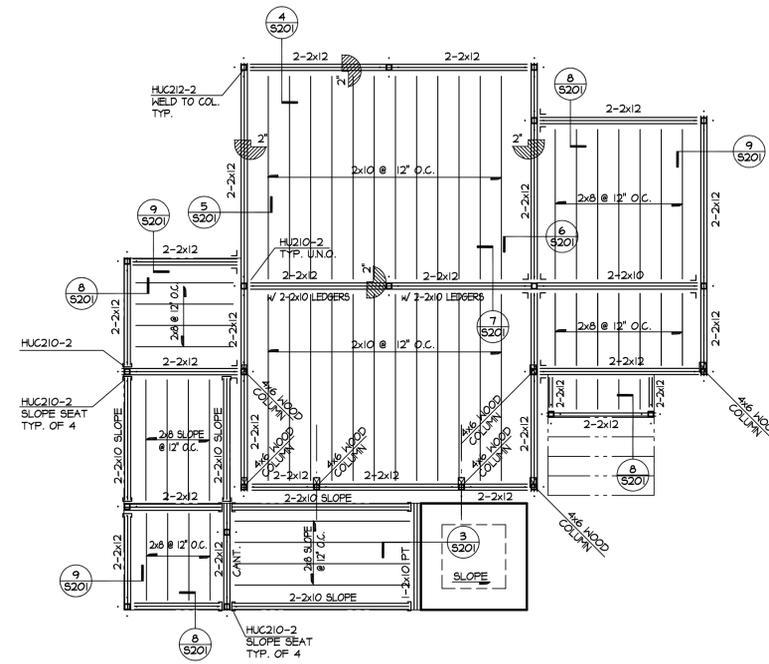
- NOTES:
1. ALL RAFTERS SHALL BE 2x6 @ 24" O.C. UNLESS NOTED OTHERWISE - MAX SPAN 12'-0".
 2. ALL HIPS, VALLEY & RIDGE RAFTERS SHALL BE 2x8 UNLESS NOTED OTHERWISE - MAX SPAN 10'-0".
 3. LOAD BEARING WALLS INDICATED AS [Symbol] SHALL BE 2x STUDS @ 16" O.C.
 4. PROVIDE MINIMUM 3-2x STUDS BELOW ALL WOOD BEAMS. PROVIDE 2-2x CRIPPLES BELOW ALL HEADERS LARGER THAN 2-2x10.
 5. HEADERS IN 2x4 LOAD BEARING WALLS SHALL BE 2-2x8 UNLESS NOTED OTHERWISE. HEADERS IN 2x6 LOAD BEARING WALLS SHALL BE 3-2x8 UNLESS NOTED OTHERWISE



FOUNDATION PLAN

SCALE: 1/4"=1'-0"

- NOTES:
1. VERIFY ALL DROPS AND DROP LOCATIONS WITH ARCHITECTURAL PLANS.



FIRST FLOOR FRAMING PLAN

SCALE: 1/4"=1'-0"

- NOTES:
1. VERIFY ALL DROPS AND DROP LOCATIONS WITH ARCHITECTURAL PLANS.
 2. ALL WOOD THIS LEVEL TO BE PRESSURE TREATED.

Seal:



Mobile Loaves & Fishes

**Community
First! Village -
Kitchens -
Phase 3 -
Neighborhoods
8 & 9**

9116 Hog Eye Rd.
Austin, TX 78724

PLAN NORTH



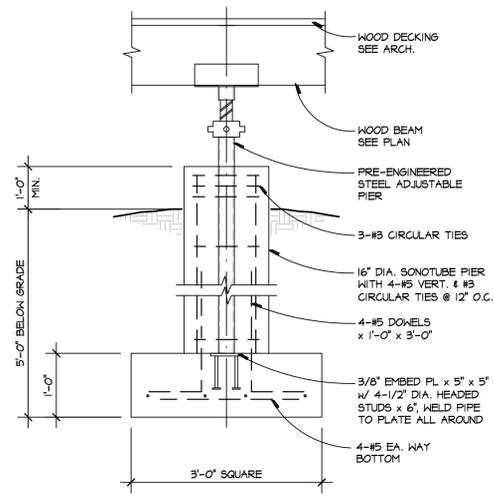
Issue

01.15.25

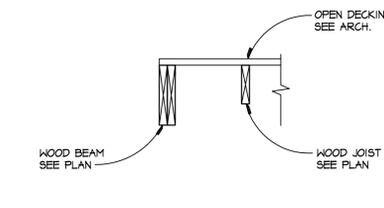
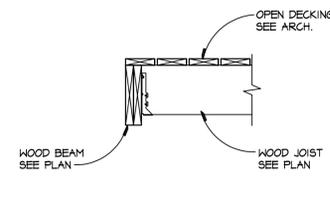
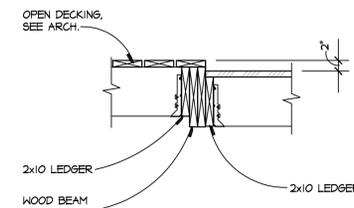
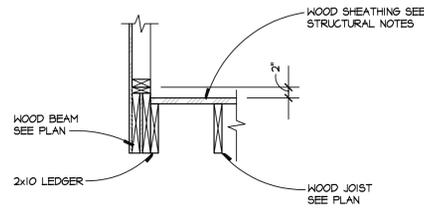
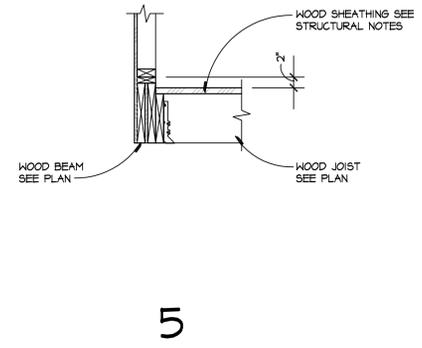
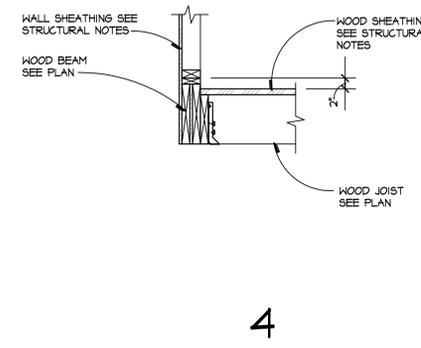
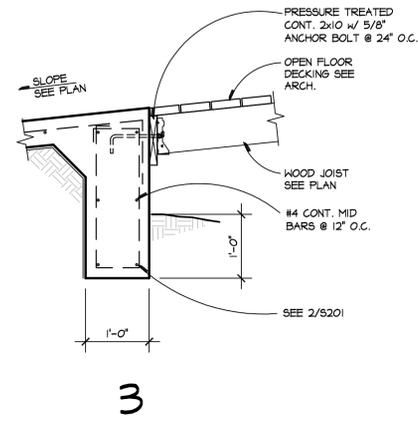
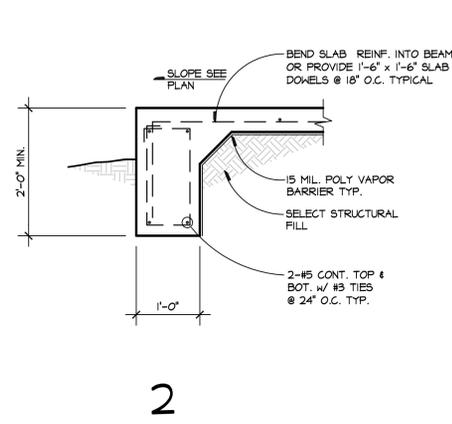
ISSUE FOR
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Project Number: 24-093a

FOUNDATION AND FRAMING
PLANS



SCALE: 3/4"=1'-0" (TYPICAL)
(UNLESS NOTED OTHERWISE)

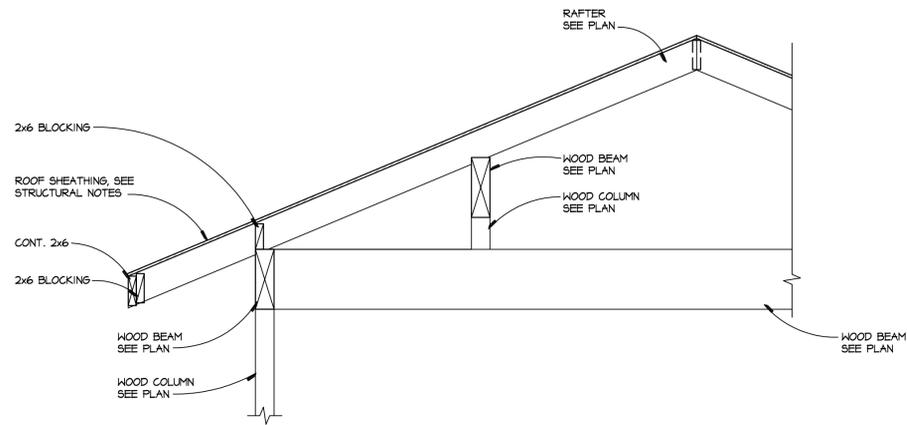


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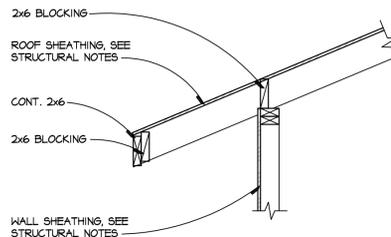
7

8

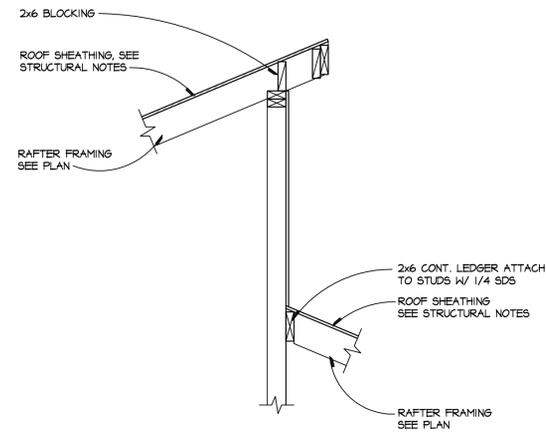
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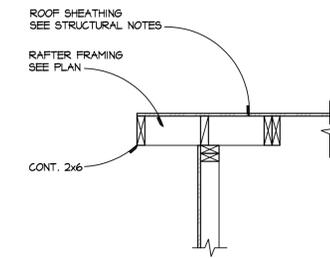
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11

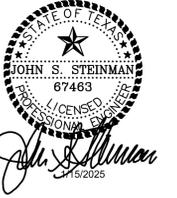


12



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Seal:



Mobile Loaves & Fishes

Community
First! Village -
Kitchens -
Phase 3 -
Neighborhoods
8 & 9

9116 Hog Eye Rd.
Austin, TX 78724

Issue
01.15.25 ISSUE FOR
CONSTRUCTION

Project Number: 24-093a

FOUNDATION AND
FRAMING DETAILS

S201