

DATE: October 15, 2026

ADDENDUM NO.: 02

PROJECT NO.: 2606

PROJECT: A New Branch Office
Johnson City Branch
622 N. Nugent Avenue
Johnson City, Texas 78636

OWNER: Security State Bank and Trust
201 W. Main
Fredericksburg, Texas 78624

ARCHITECT: Chesney Morales Partners, Inc.
4901 Broadway Suite 250
San Antonio, Texas 78209



10/15/2626

This addendum is generally separated into sections for convenience; however, all contractors, subcontractors, material men, and all other parties shall be responsible for reading this entire addendum. The failure to list an item or items in all affected sections of this addendum does not relieve any party affected from performing as per instructions, providing that the information is set forth one time any place in this addendum. The addendum forms a part of the Contract Documents, modifying and superseding where it is inconsistent with them. All other conditions of the Contract Documents remain unchanged.

CLARIFICATION:

- ITEM 1 **Allowance No. 1**
Allow the sum of \$8,000 (Eight Thousand Dollars) for the purchase of each ring light fixture shown on the Architectural ceiling plan. Note: power, controls, installation, and labor to be included in the contract cost.
- ITEM 2 See attached list of questions and responses
- ITEM 3 See attached revised sheets
Sheet A1.3
Sheet A3.4
Sheet A3.5
Sheet A6.1
Sheet A6.3
Sheet S1.1
Sheet S2.1
Sheet S2.2
Sheet E501

END OF ADDENDUM NUMBER 2

Addendum No. 02

Responses to Questions form Bidders:

Question 01 - Is the intent to award the project based on the lowest qualified bid, or will experience, qualifications, schedule, and the proposed project team also be considered? Also, how many contractors have been invited to bid?

Reply: This is an invited group of 5 bidders selected by SSBT based on experience and qualifications. We have stated that the contractor will be selected on the basis of cost and construction duration, however, it is the Owner's purgative if they want to discuss or meet with the contractor(s) and their project team (principal in charge, superintendent, project manager) to make the final decision.

Question 02 - What is the anticipated construction start date and desired completion date?

Reply: The construction duration is to be determined by the bidders but will be an equal consideration in determining the successful bidder.

Question 03 - Has the project been approved to move forward with construction, or is this bid request primarily for budgeting purposes?

Reply: This is not a Pricing exercise . We are close to having all permits in hand. Building and Drainage are complete; we are waiting on TxDot and the final site plan approval. We hope that they will be in hand around the same time or shortly after the Contract is signed

Question 04 - I see Mecho Shades called for on A6.2. They are called out to be manual roller shades on 6/A8.1 and in the finish schedule on A11.1

Note 15 on E301 calls for motorized shade controllers, Circuit 20 on panel A2 is for motorized shades. Could you please clarify if manual or motorized roller shades are required for this project?

Reply: Both motorized and manual roller shades will be used in this project. Please refer to the plans for the location of each type.

Question 05 - The spec book says that division 12 - furnishings is not used.

Reply: Correct

Question 06 - Will the Vault and Drive Thru Window, window Type O, be purchased by owner or by contractor?

Reply: Both BR teller window and the vault will be purchased by the Owner and installed by the Owner

Question 07 - On the door hardware schedule, hardware sets 1 and 2 call for an LCN concealed auto operator. The Door hardware specifications have a surface mounted door opener listed. The LCN concealed auto operators will not fit within the storefront frame. Would a Stanley M-Force surface mount Automatic Door Operator be acceptable for these doors?

Reply: The LCN concealed auto opener is not intended to be concealed in the storefront frame. Please refer to the door type schedule and the wall sections in the document that shows aluminum header above the door.

Question 08 - Can you look at drawing S-2.2, detail F

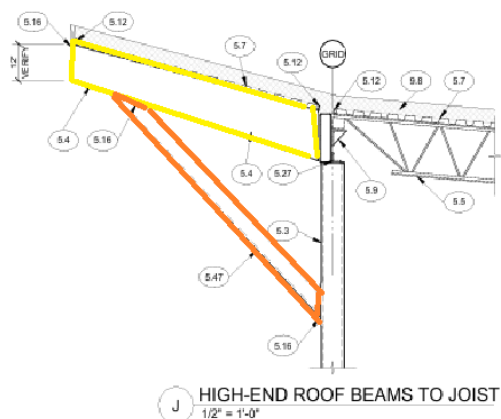
It is calling out for item 5.77, which under keynotes item 5.77 states the following: HSS___x___x___. What size is the HSS, please advise.

Reply: The steel tube size that should be noted in Keynote 5.77 is an HSS6x2x1/4 rectangle tube laid flat. This beam is to catch the lower roof joists since they come in slightly above the lower beam carrying the mechanical platform

See attached sheet S-2.2

Question 9 - Drawing S-1.4, left side of grid E shows a steel beam which is exposed Drawing S-2.1, details E & J, show a better view. The beam has a HSS 6x6 kicker under it What is this beam,? Is it a beam, channel or HSS?

Also the HSS 6x6 diagonal kicker, item 5.47 is not sized for the thickness (HSS 6x6x?)



Reply:

9a: The diagonal kicker is HSS6x6x1/4 please.

9b: On the tapered beam, this member being flush with the width of the column and diagonal kicker for smooth interfaces between the member intersections. Since we have an HSS8x6 column at these locations with the 6" side facing out, set this tapered beam to be 6" wide. Then we have it intersecting a 16" deep tube on the front upper edge of the building but AISC doesn't make HSS16x6 tubes so we would need to use a SS18x6x3/16 steel tube with the bottom edge cut at the required dimensions and tapered as shown.

This will be more than sufficient for this condition. **See attached sheet S2.1**

Question 10 - Lobby 101 is shown to receive the WD1 acoustic panels. The finish schedule calls for this ceiling to be sloped following the roof structure. The cut section in this area, 1/A3.4, does not show the ceiling panels. Should we assume that these ceiling panels will be fastened directly to the bottom of the metal deck?

Reply: See attached sheets A3.4, A3.5, A6.3

Question 11 - Would schedule 40 PVC pipe and fittings be acceptable for the sanitary in lieu of hubbed cast iron below grade and no-hub cast iron above grade?

Reply: Acceptable

Question 12 - Would Uponor pex pipe and fittings be acceptable for domestic water in lieu of type L hard copper? Of course we would use straight sticks in track for rigidity.

Reply: Pex pipe is NOT accepted

Question 13 - Looking at the Toilet Accessories Schedule on Page A4.1.

TA-4 Paper Towel Dispenser Lists Model #35903. The Specs for Toilet accessories Lists Model # 4369. Please Advise which is correct?

Reply: Model # 35903 is the correct model

Question 14 - No Model Number is Provided for the Mirrors. Please Provided Model Number.

Reply: Bobrick B-209 2472

Question 15 - Will Fire Alarm or Fire Sprinkler systems be required on this project?

Reply: The project is not required to be fully sprinklered. A fire alarm system is not required for the branch bank. The HVAC equipment schedules include a note to require smoke detectors in the units that are large enough to require it. Those smoke detectors are intended to be stand alone to handle the required unit shutdown and annunciation.

Question 16 - Please confirm the discrepancy between the Structural notes (3.5' of select fill) and the Geotechnical report (6'-0" to 7'-0" of select fill).

Reply: See attached sheet S-1.1

Question 17 - For the movement sensitive flatwork mentioned in the geo-tech and structural notes, we are assuming the wide sidewalks to the left and in front of the building meet this definition. Please confirm these are the only movement sensitive sidewalks or flatwork areas.

Reply: Yes, correct. Extend the select fill to the curb.

Question 18 – Substitution Request: UnaClad UC-4 Metal Roof Panels

Reply: Acceptable. Color to match Berridge Lead Cote

Question 19 – Substitution Request: UnaClad UC-500 Soffit Panels

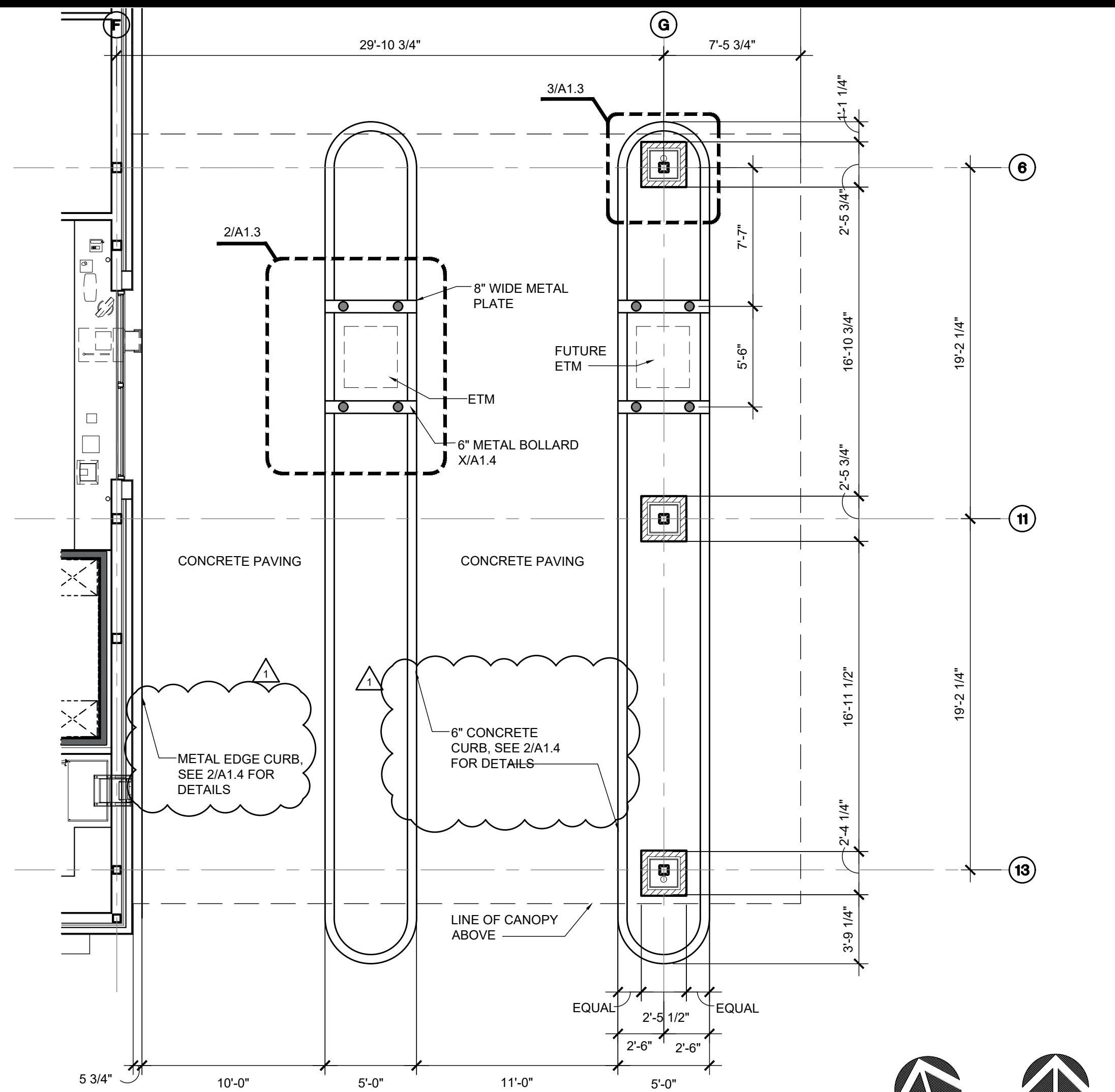
Reply: Physical sample is required and to be reviewed by owner before acceptance

Question 20 – Substitution Request: UnaClad Metal Wall Panels

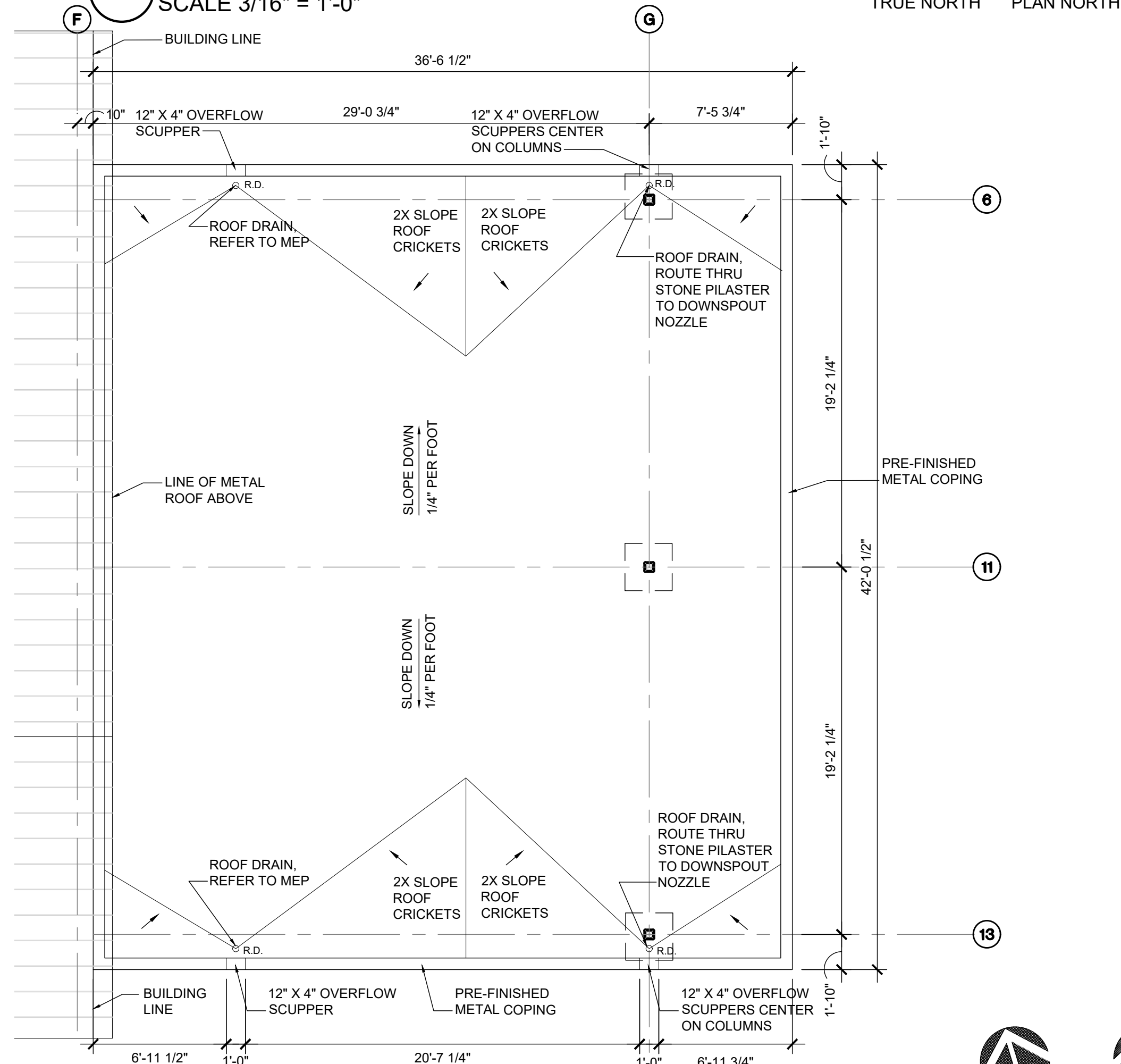
Reply: Acceptable

Question 21 – Substitution Request: OX Engineered Insulation

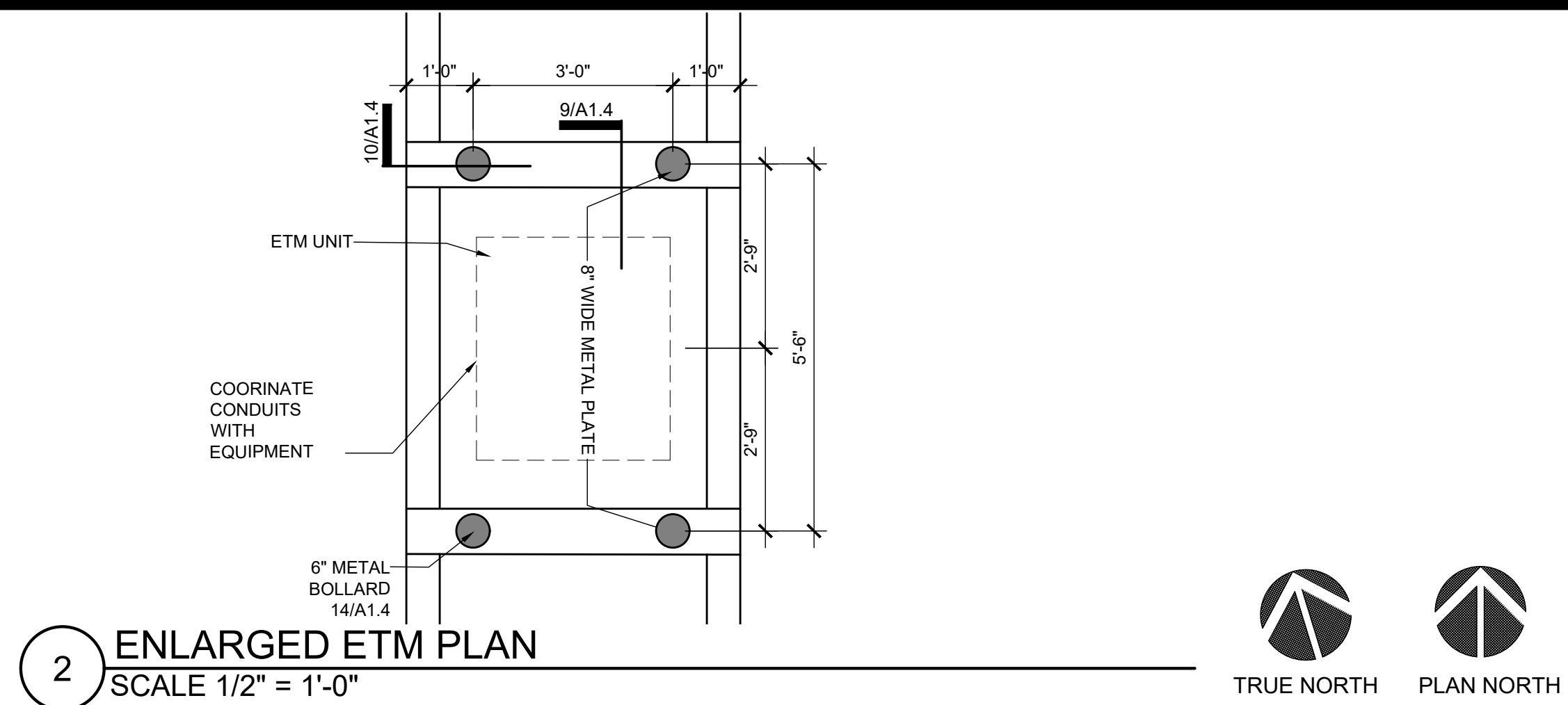
Reply: Acceptable



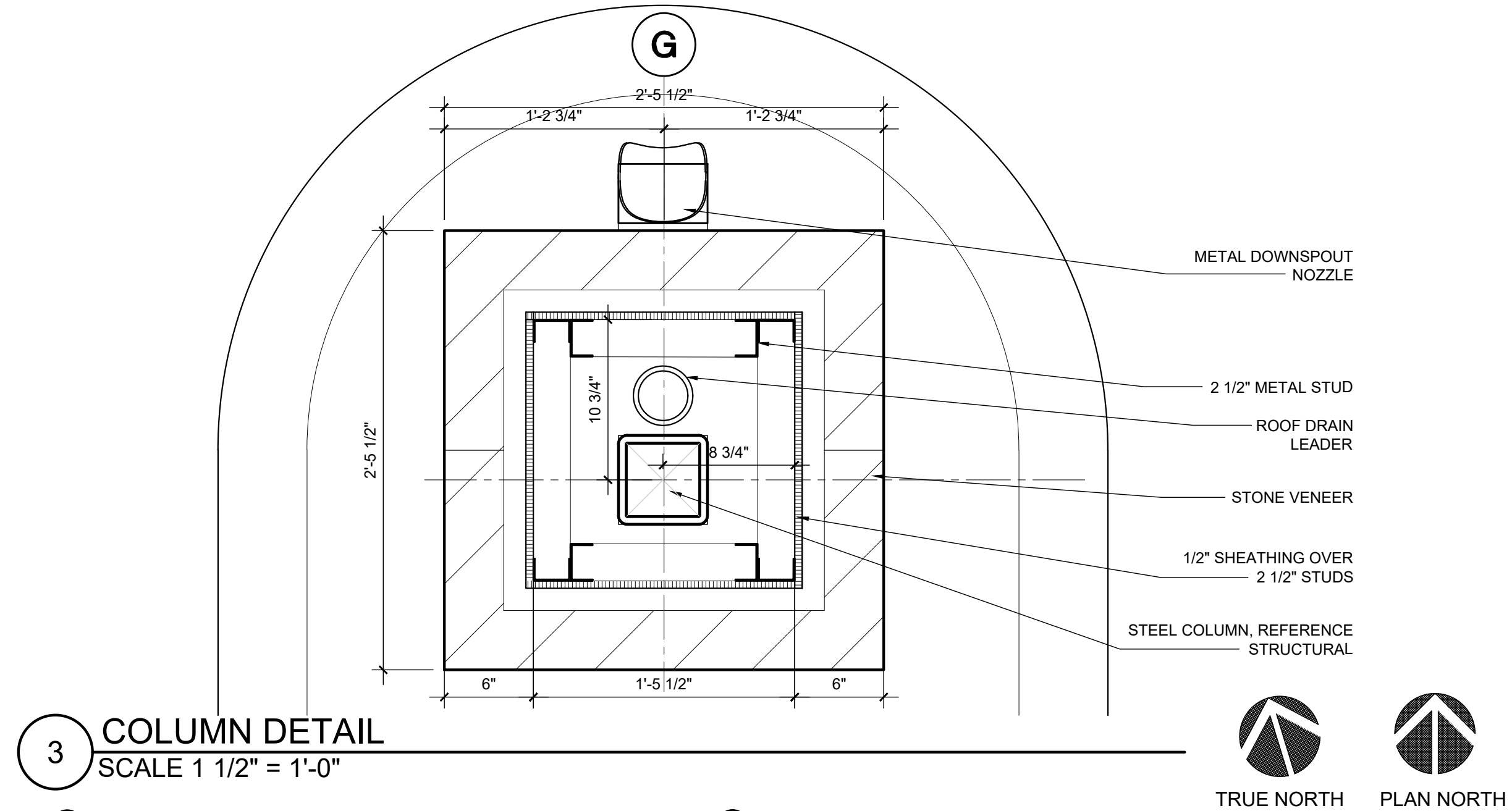
1 DRIVE THRU CANOPY PLAN
SCALE 3/16" = 1'-0"



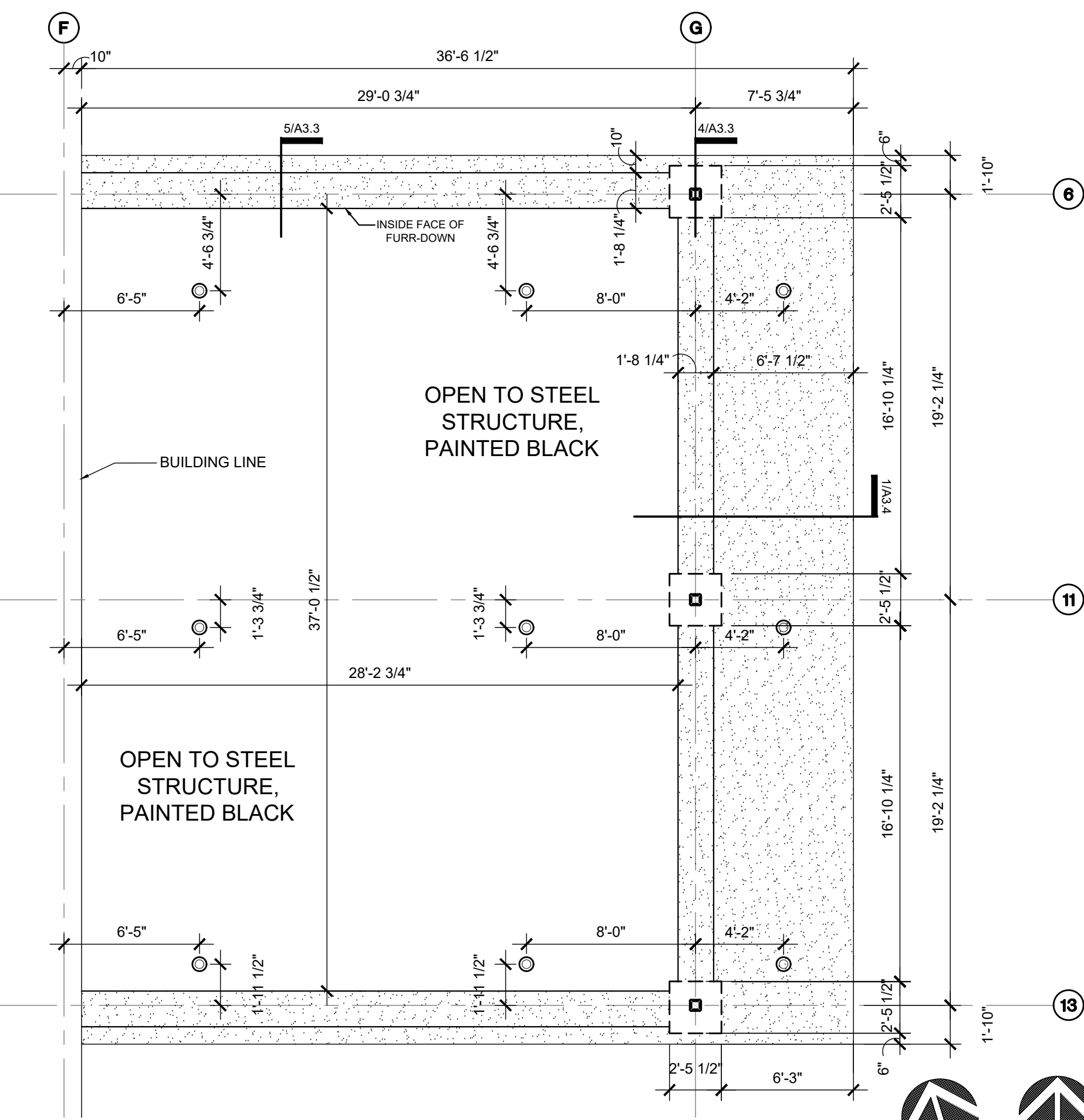
4 DRIVE THRU CANOPY ROOF PLAN
SCALE 3/16" = 1'-0"



2 ENLARGED ETM PLAN
SCALE 1/2" = 1'-0"



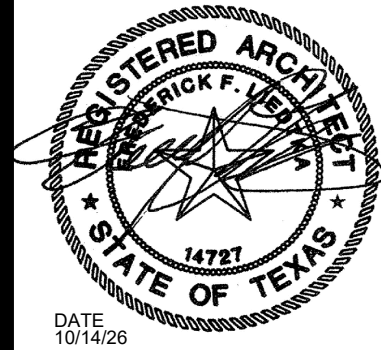
3 COLUMN DETAIL
SCALE 1 1/2" = 1'-0"



5 DRIVE THRU REFLECTED CEILING PLAN
SCALE 3/16" = 1'-0"

DRIVE THRU
CANOPY

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622 N NUGENT AVE
JOHNSON CITY, TX 78636
**SECURITY STATE
BANK & TRUST**

chesneymoralespartners, inc.
architecture | interior design
4901 Broadway, Suite 250 | San Antonio, Texas 78209
210.828.9481 | TBAE Firm BR1010
cmp

JOB NO: 2606
DATE: 08-11-26
DRAWN BY: MDJ
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SHEET

A1.3



DATE
10/14/26

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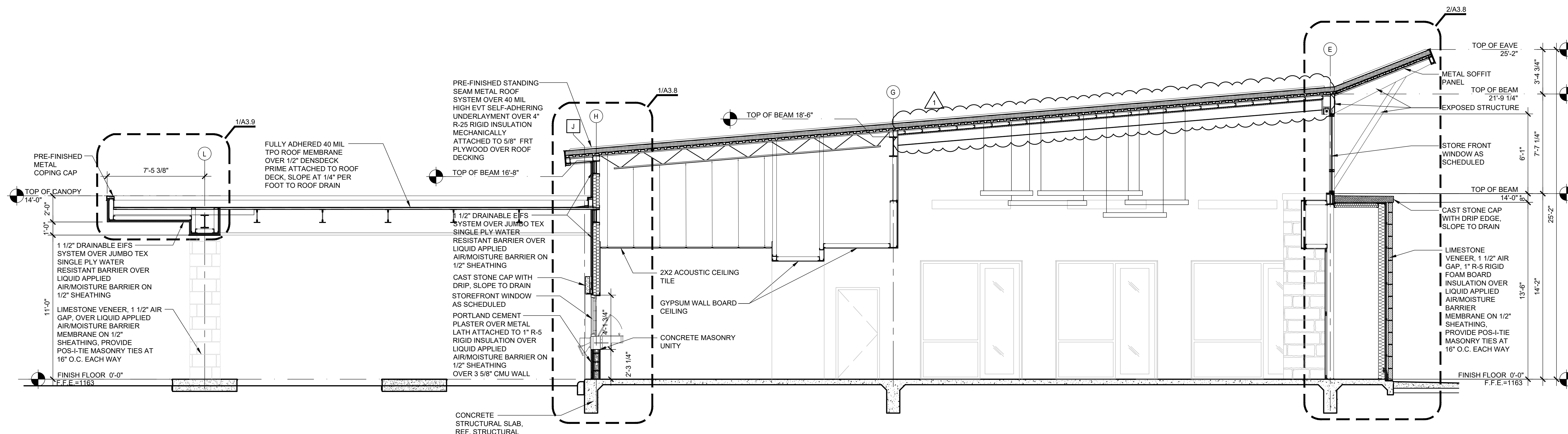
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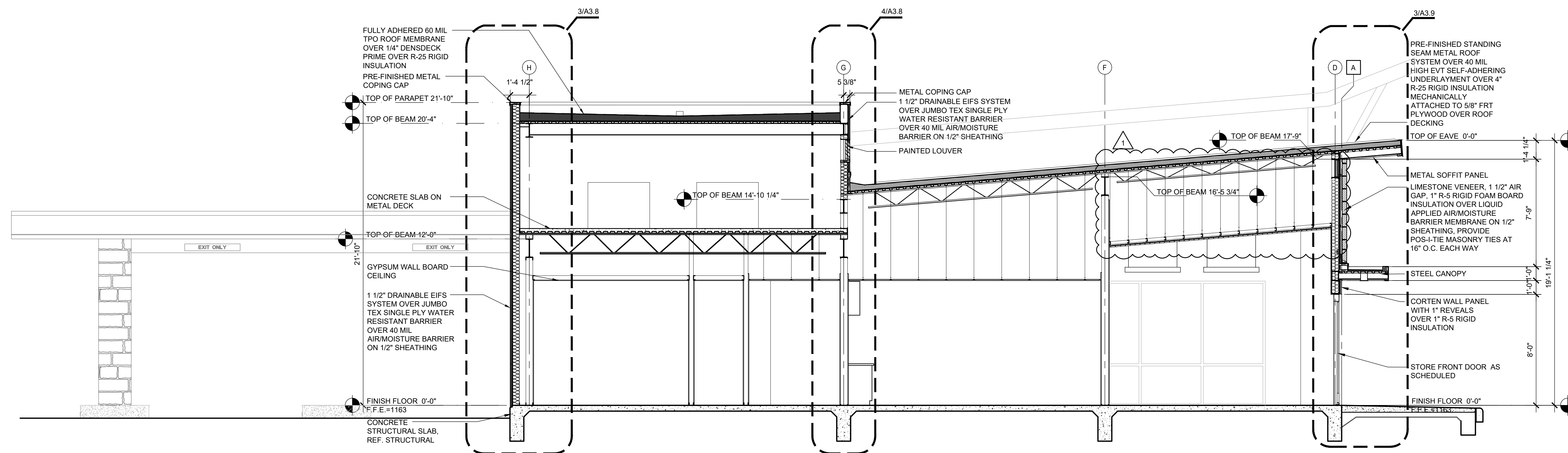
CHECKED BY: FFL

SHEET

A3.4



1 BUILDING SECTION
SCALE 1/4" = 1'-0"

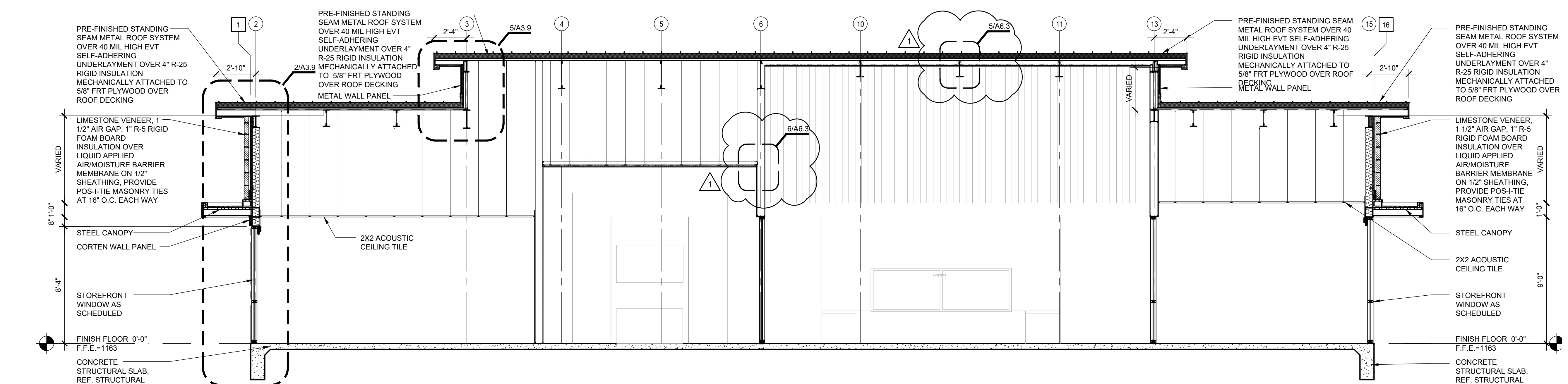


2 BUILDING SECTION
SCALE 1/4" = 1'-0"

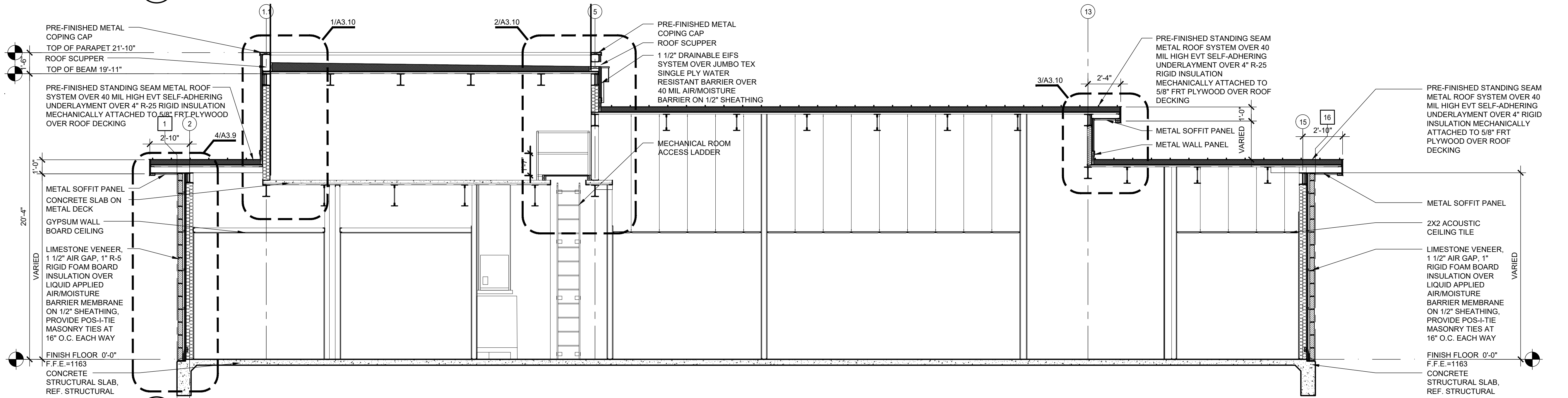
- NOTES:
1. EXTERIOR WALLS TO HAVE GYPSUM SHEATHING
 2. PROVIDE FIRE RETARDANT TREATED (FRT) PLYWOOD AT ROOF DECK
 3. ALL CONTINUOUS WOOD BLOCKING AT ROOF AND EXTERIOR WALLS TO BE FIRE RETARDANT TREATED (FRT), PRESSURE TREATED BLOCKING

BUILDING SECTIONS

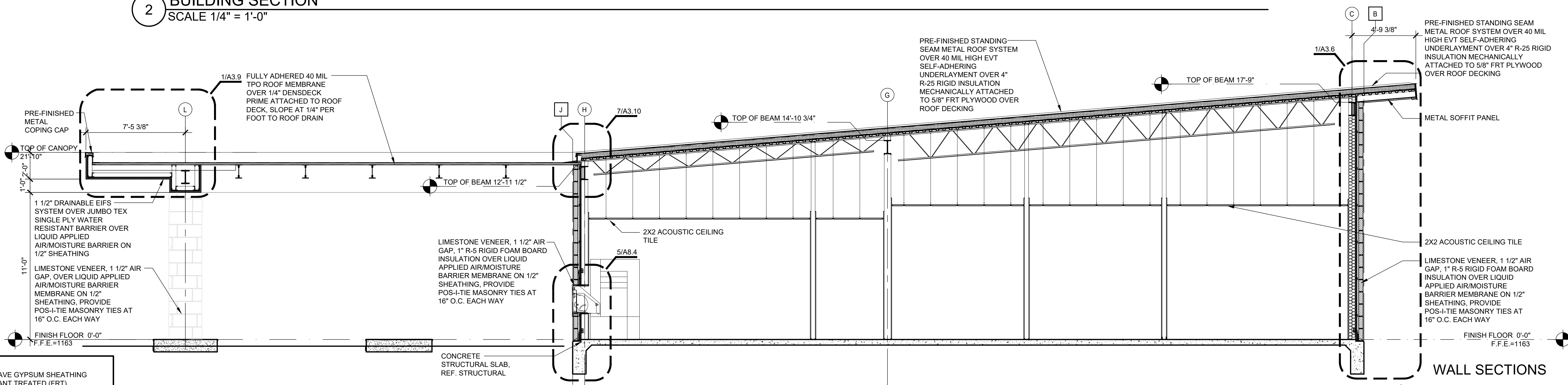
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1 BUILDING SECTION
SCALE 1/4" = 1'-0"



2 BUILDING SECTION
SCALE 1/4" = 1'-0"



WALL SECTIONS

- NOTES:
1. EXTERIOR WALLS TO HAVE GYPSUM SHEATHING
 2. PROVIDE FIRE RETARDANT TREATED (FRT) PLYWOOD AT ROOF DECK
 3. ALL CONTINUOUS WOOD BLOCKING AT ROOF AND EXTERIOR WALLS TO BE FIRE RETARDANT TREATED (FRT), PRESSURE TREATED BLOCKING

3 BUILDING SECTION
SCALE 1/4" = 1'-0"



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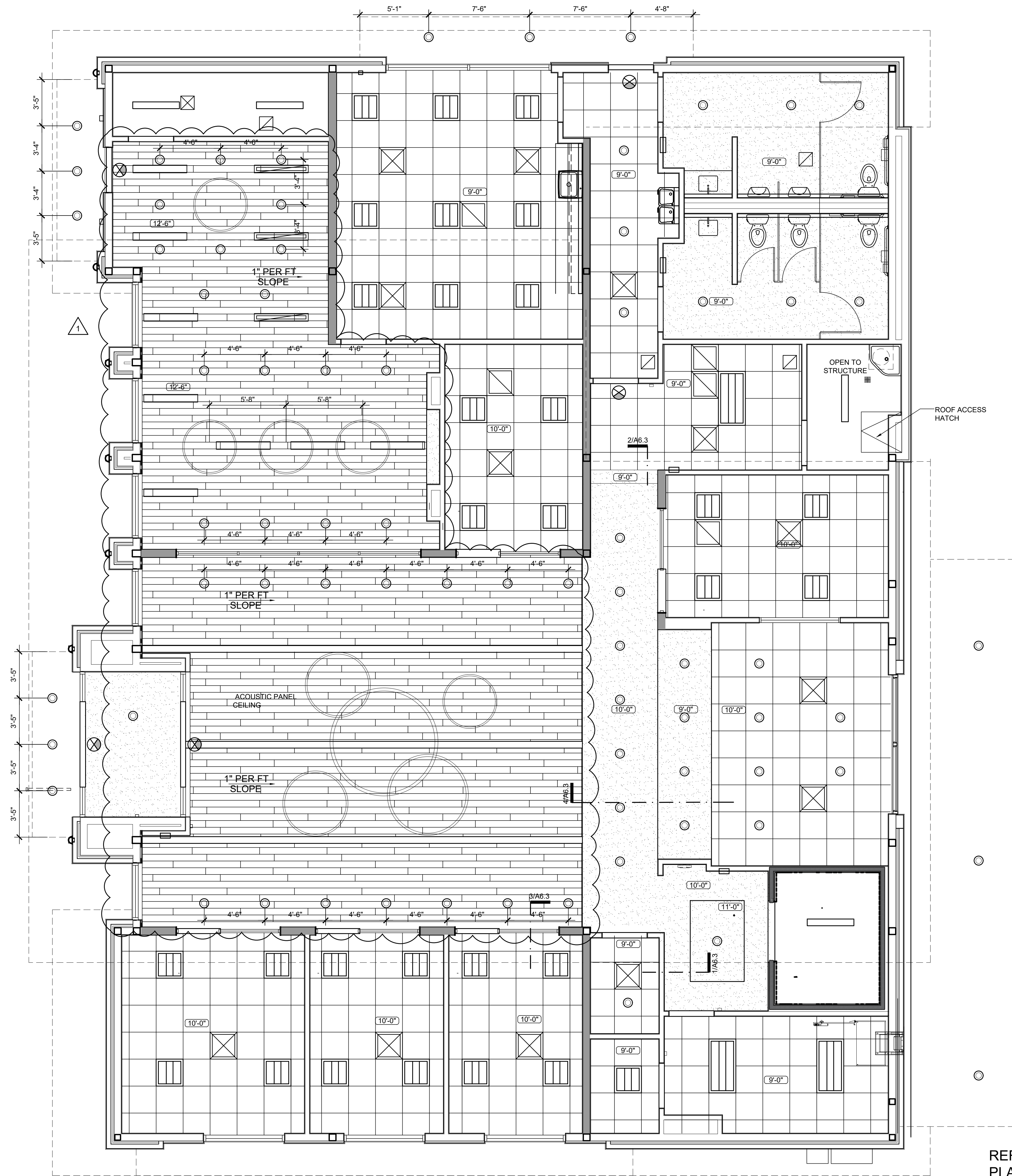
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A3.5

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CEILING PLAN LEGEND			
	2'-0" X 4'-0" SUSPENDED ACOUSTICAL TILE CEILING		2'-0" X 4'-0" LAY - IN LIGHT FIXTURE
	2'-0" X 2'-0" SUSPENDED ACOUSTICAL TILE CEILING		2'-0" X 2'-0" LAY - IN LIGHT FIXTURE
	GYPSUM WALLBOARD CEILING		RECESSED CAN LIGHT FIXTURE
	ARCHITECTURAL LIGHT FIXTURE		SUSPENDED LIGHT FIXTURE
			WALL MOUNTED LIGHT FIXTURE
			EXIT LIGHT
			SUPPLY AIR VENT SEE MEP DRAWINGS
			RETURN AIR VENT SEE MEP DRAWINGS
			SUPPLY AIR VENT SEE MEP DRAWINGS
			RETURN AIR VENT SEE MEP DRAWINGS
			PLASTER CEILING
			WALL PACK



1 REFLECTED CEILING PLAN
SCALE 1/4" = 1'-0"



REFLECTED CEILING
PLAN

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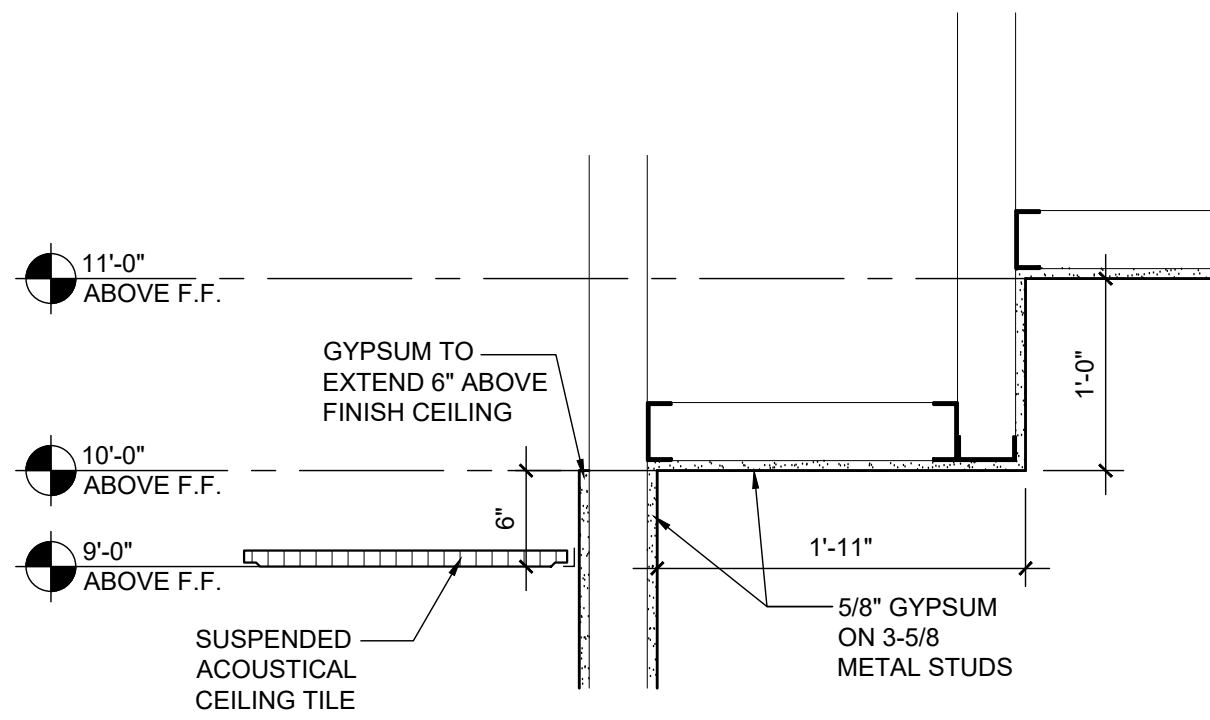
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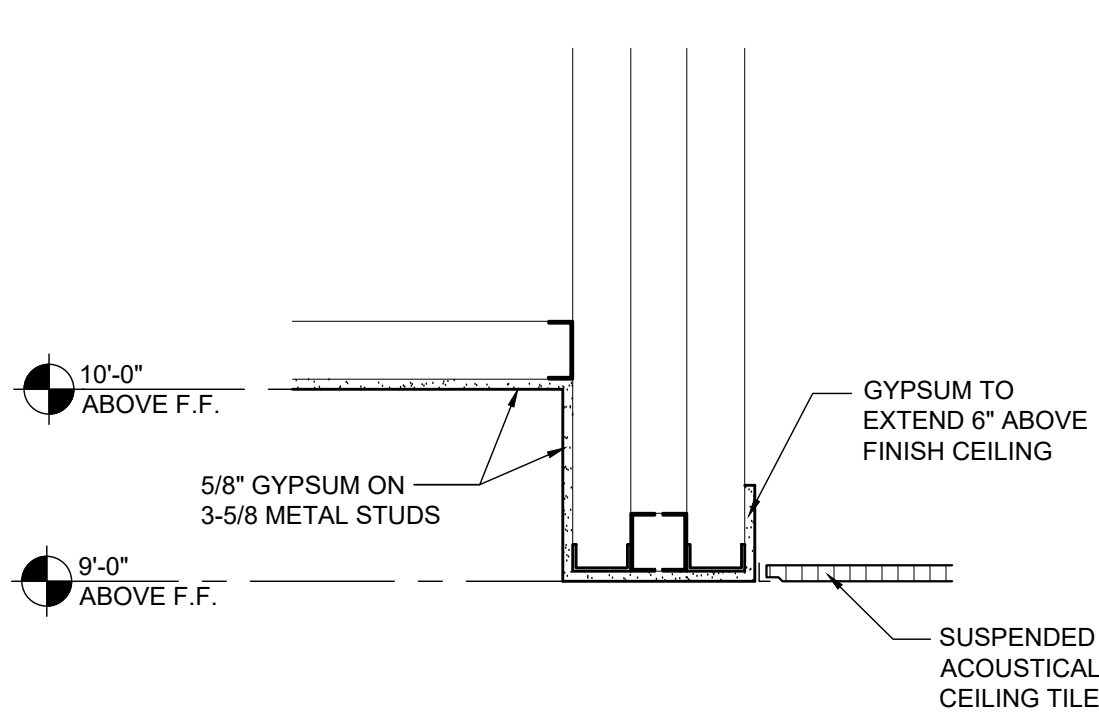
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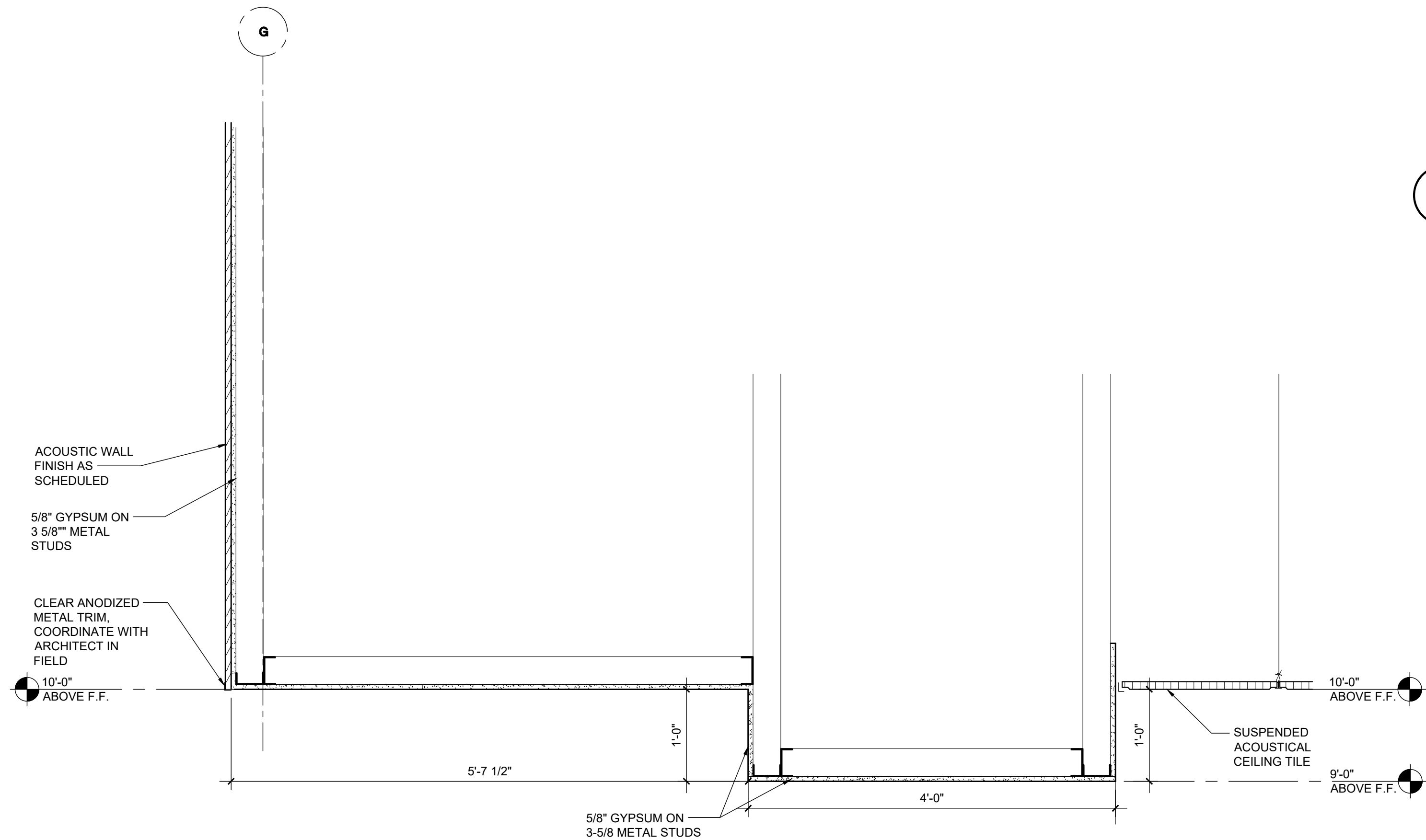
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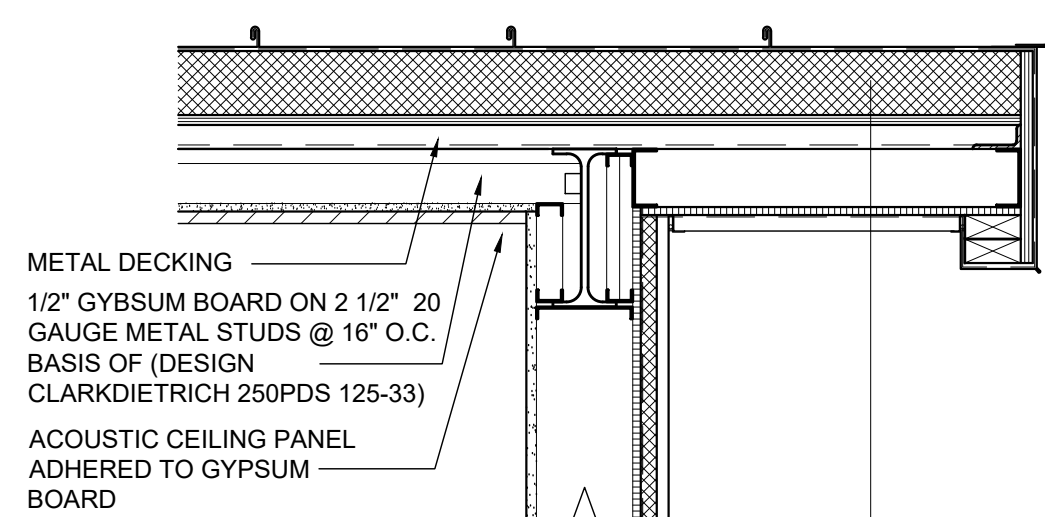
1 FURR DOWN DETAIL
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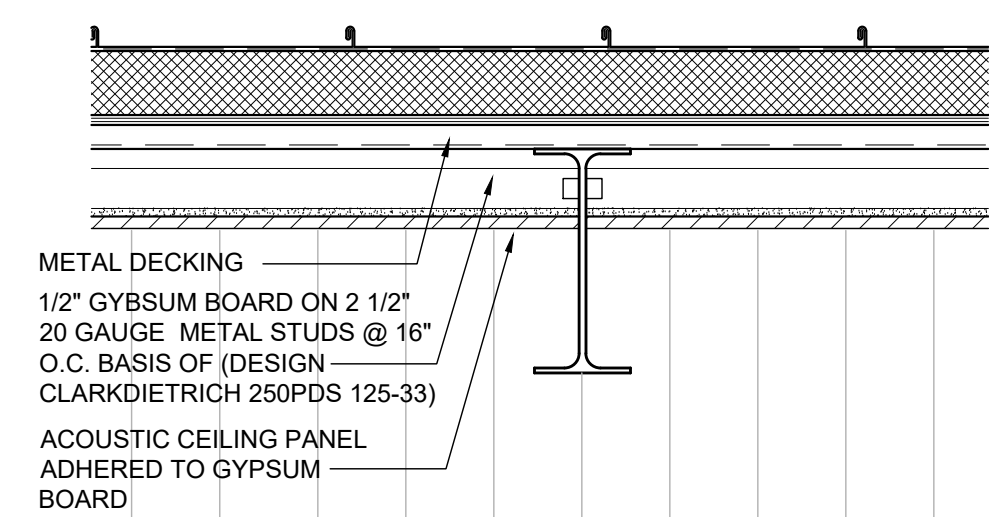
2 FURR DOWN DETAIL
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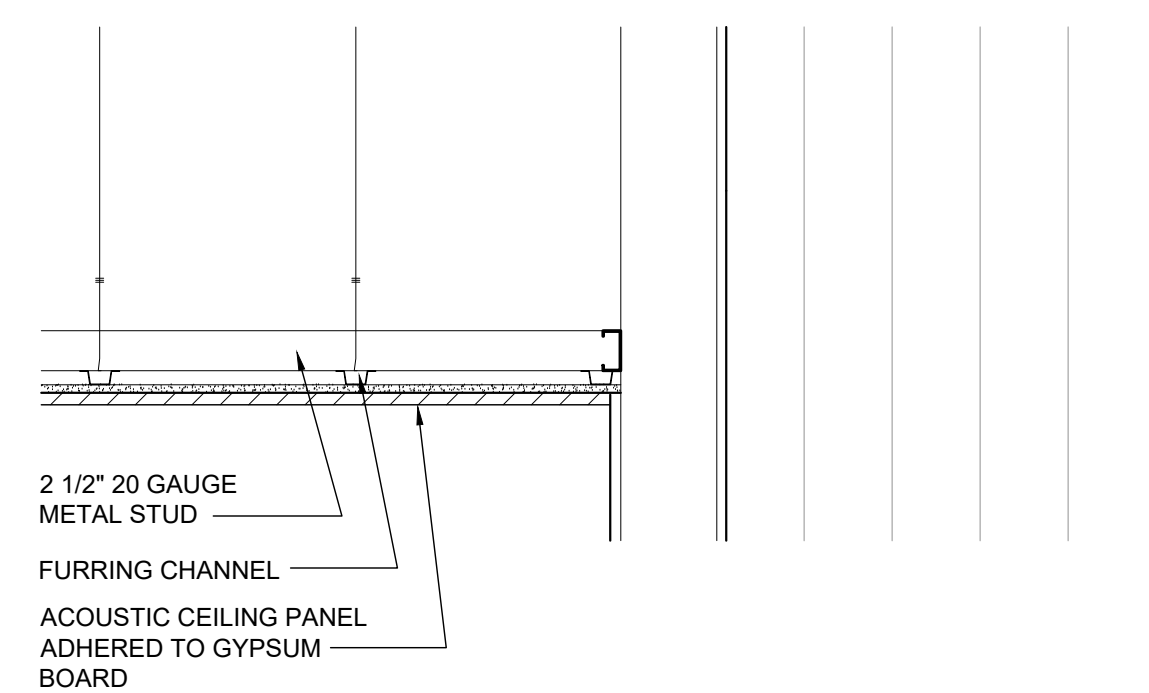
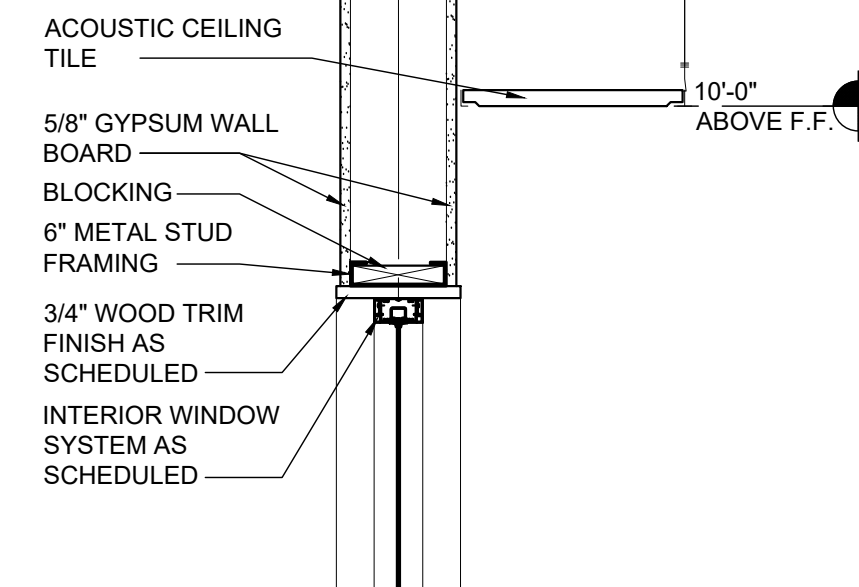
4 FURR DOWN DETAIL
SCALE 1" = 1'-0"



3 CEILING DETAIL
SCALE 1" = 1'-0"



5 CEILING DETAIL
SCALE 1" = 1'-0"



6 CEILING DETAIL
SCALE 1" = 1'-0"



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A6.3

Structural Design Criteria

- The 2015 International Building Code is the basic code document used in the preparation of these structural documents. Additional codes and references are as noted. All structural work shall be according to all local codes in addition to this basic code document.
- The structural engineer-of-record prepared specifications for structural related portions of the project and has included these specifications on the structural drawings and/or in the project manual. Specifications for other consultants are included in the project manual.
- Wellborn Engineering & Surveying investigated the subsurface soils and provided foundation design recommendations. (Report Number 26-084-01, dated 05/27/2026) Axis Structural LLC based the foundation design upon this information.
- The Contractor shall familiarize himself with the site and subsurface conditions. Ignorance of conditions is not a basis for a claim for additional compensation. Layout the building by a licensed surveyor.
- All contractors bidding the project shall review all plans, details, and detail keynotes to include all items in their bid that may only be shown on the plans or only shown in the details. Missing members in a bid that are shown only in the details or keynotes, but not on the plan, is not a basis for a claim for additional compensation if these items are missed in the original bid. Contractor shall also review all other consultant drawings in case there are additional materials/services they might be required to provide that are not shown on the structural drawings.
- Drawings of specific details in the structural documents indicate the intent of the structural design and in most cases, are typical conditions or very similar to other details. Consider typical conditions, not necessarily noted as typical, as typical for other conditions.
- Understanding the structural requirements shown on the structural documents requires cooperation among all parties involved. Design and construction are complex. Although Axis Structural LLC designed the project with due care and diligence, we do not guarantee perfection. Communication is necessary. Immediately report structural discrepancies to Axis Structural, LLC for review and correspondence. Consider unresolved discrepancies as the most costly solution of the discrepancy.
- The structural documents represent the finished structure. Total project definition (and therefore the definition of all requirements) will be provided by combining all documents with the structural documents. The Contractor shall verify all field conditions which will affect the fabrication of components for new construction prior to the start of construction. Unless otherwise indicated, the documents do not indicate the method of construction. The Contractor shall take all measures necessary to protect the safety of the public along with the safety of the structure during construction. Such measures shall include but not be limited to bracing and shoring of dead loads, construction loads, and wind loads. The contractor shall comply with OSHA Excavation and Trench Safety Standards. The contractor will be required to correct at his/her own expense any subsidence structural damage or other objectionable conditions caused by his/her operations.
- General contractor is responsible for coordinating all site visits noted in the different sections of the structural notes and is responsible for requesting all documents required by the city/county in a timely manner for project permit close out. General Contractor is also responsible for obtaining any/all Special Inspection documents per Chapter 17 of the referenced IBC from the Special Inspector and RDPIRC and submit to the city/county reviewer as required. Final engineer completion letters for the project will not be provided if consultant fees are not paid in full at the time the request is made. Allow at least 72 hours for letters to be written after requests are made.
- The Texas Architectural Barriers Act (Article 9102, Texas Civil Statutes) requires the prime design consultant to submit construction documents for all projects with an estimated construction cost of \$50,000 or more to the Texas Department of Licensing and Regulation for review before commencing construction. Failure to comply may result in up to \$1,000 per day administrative penalties for each violation. The Owner and prime design consultant understand Axis Structural LLC will perform only structural engineering services, which does not include submitting documents to the Texas Department of Licensing and Regulation.
- Structural design is based on the following:
Floor live loads = Mechanical rooms = 125 psf
Floor live loads shall be posted for each floor or part thereof
Floor dead load = 58 psf

Roof live load = 20 psf, Tributary area considered, Ponding not considered
Roof dead load plus finish out collateral load = 20 psf (mechanical unit wts not included in this load)

Structural framing is designed only for the total gravity (static) loads indicated above. Acoustic and dynamic loading from mechanical equipment including induced vibrations is not considered.

Refer to roof framing plan and mechanical plans for weights and locations of roof top mounted mechanical equipment.

Snow Loads
Ground Snow load = 5 psf

Wind Loads
ASCE 7-10 Chapter 27 Part 2 - Enclosed Simple Diaphragm Buildings with H<=160'

Ultimate design wind speed (3-second gust) = 115 mph
Nominal design wind speed (3 sec. gust) = mph
Structure Type = Building
Structure Risk Classification Category II, Exposure Category C
Topographic Effects (Kzt) = 1.0, Gust Effect Factor (G) = 0.85, Rigid Structure
Enclosure Classification: Enclosed
Internal Pressure Coefficient: +/-0.18
Importance Factor: 1.0
Reliable dead load for net uplift calculations = 5 psf

Components and Cladding Wind Pressures (1.0W)

Roof	Surface Pressure (psf)
Area	10 sf 100 sf >500 sf
Negative Zone 1	-39.1 -39.1 -39.1
Negative Zone 1'	-54.4 -51.3 -51.3
Negative Zone 2	-45.2 -42.1 -42.1
Negative Zone 3	-60.5 -42.1 -42.1
Positive All Zones	16.0 16.0 16.0

Wall	Surface Pressure (psf)
Area	10 sf 100 sf >500 sf
Negative Zone 4	-35.7 -30.9 -27.5
Negative Zone 5	-44.0 -34.3 -27.5
Positive Zone 4 & 5	33.0 28.1 24.7

Edge strip dimensions, 0.2h = 48", 0.6h = 144". Perimeter and Corner zone dimensions related to these edge strip dimensions are shown in ASCE7-10 Chapter 30.

Earthquake
Risk Category = II
Seismic Importance Factor = 1.0
Spectral response acceleration parameters
Ss = 0.061
S1 = 0.031
Site Soil Class = D
Design spectral response acceleration parameters
Sds = 0.041
Sd1 = 0.02
Seismic Design Category = B
Basic Seismic-Force-Resisting System = Concentric steel braced frames
Response Modification Factor(R) = 3.0
Seismic Response Coefficient(Cs) = 0.022
Analysis Procedure = Simplified
Design Base Shear = 0.022W

Soil Design Parameters: (from geotechnical engineer recommendations)
The soils supporting the foundation are improved so that the classification of soils is not considered expansive (IBC 1808.6.3 and 1808.6.4)

Improved PVR = 1 inch
Frost depth below final grade is not applicable
Min ext beam depth below final grade = 18"
Allowable soil bearing capacity (Fp) = 2,500 psf
Coefficient of Subgrade Friction = 0.35

Structural Submittals

Submit to the Structural Engineer for review appropriate schedules, shop drawings, samples, test reports, and product data that is related to the structural portion of the Work according to AIA Document A201 General Conditions of the Contract for Construction. No work shall be fabricated until Structural Engineer's review has been obtained. A list of structural submittals required for this project is:

Deferred Structural Submittal types to be reviewed by Structural Engineer:

Vapor Retarder
Foundation reinforcing steel
Concrete mix design:
Curing compound specifications:
Prime paint specifications
Welder qualifications
Structural steel
Steel joists
Metal floor deck fabrication
Metal roof deck
Light gage metal studs

Deferred Structural submittals requiring delegated engineering design to be reviewed by Structural Engineer:

Exterior cold formed metal stud framing
Prefabricated steel joists

** Deferred submittals not specifically noted in these lists are to be reviewed by other consultants, not by structural engineer.

*** All questions related to delegated designs shall be directed to the Specialty Engineer providing the design.

Special Inspection Reports to be provided to Structural Engineer:

Earthwork below building testing reports
Concrete test results
Concrete monitoring during concrete placement reports
Bolt tightening test results
CWI Inspection Reports

Items Requiring Structural Engineer's Site Visit for IBC Chapter 1 compliance:

(**These observations do not replace the need for special inspections on these items.)

Concrete slab and footing reinforcing prior to concrete placement
- Second floor metal deck attachment prior to concrete placement
- Metal roof deck attachment prior to covering with roofing material
- Metal stud framing prior to covering
- Structural steel framing prior to covering

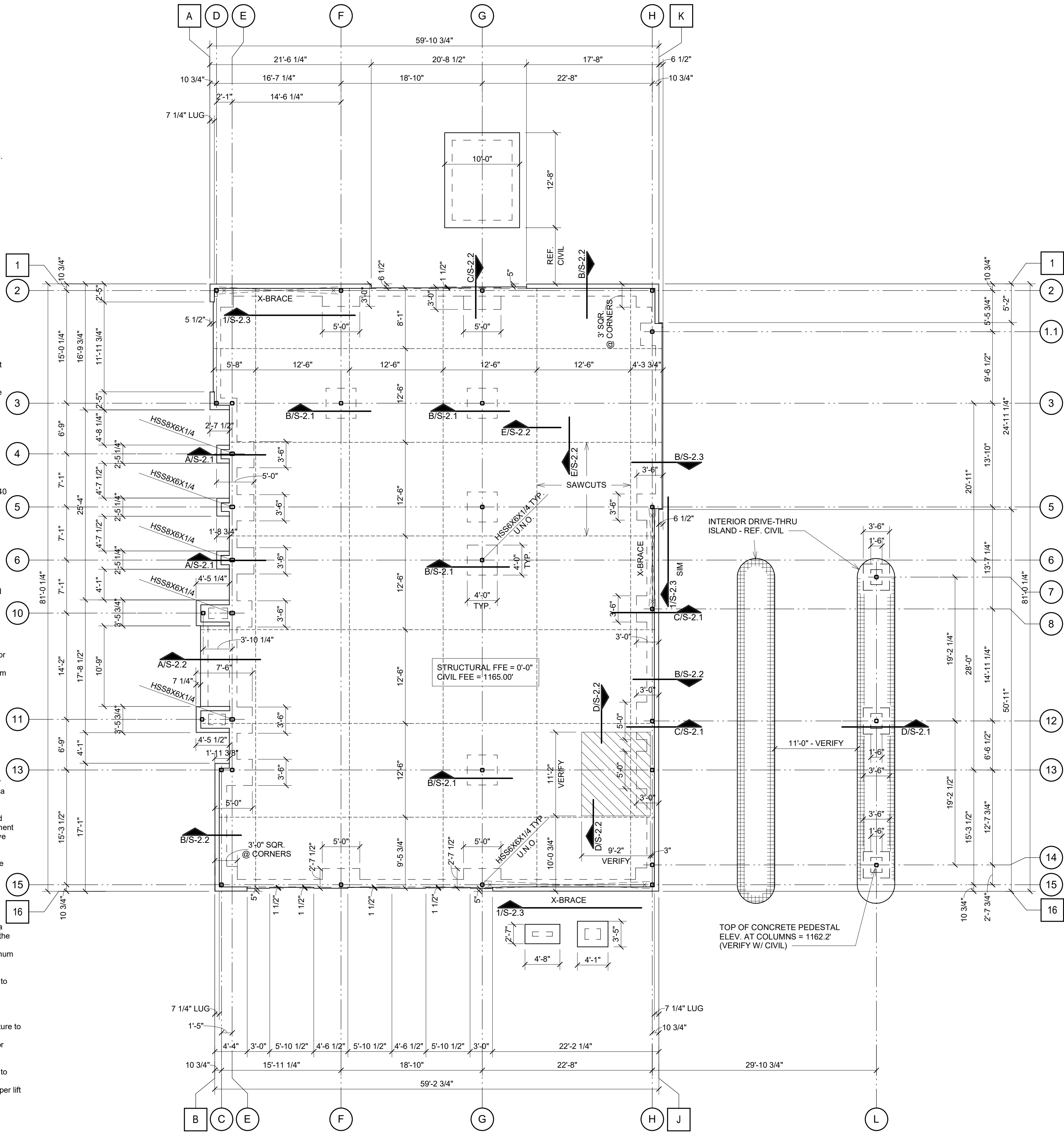
Structural Special Inspections and Testing

- The registered design professional in responsible charge (RDPIRC) for this project is the Architect. Submit all special inspection reports directly to the RDPIRC for review. Also, submit the structurally related special inspection reports to the structural engineer for his review.
- The general contractor is responsible for coordinating all testing, inspections and notifying the Architect/Engineer and Special Inspectors of work ready for inspection. The general contractor must provide access to and means for proper inspection of such work.
- A qualified independent testing laboratory (meeting the qualifications of ASTM-329) shall perform the inspection and testing services as the special inspector(s) as required by laws, ordinances, rules, regulations, orders or approvals of public authorities, and also additional inspections, sampling and testing required for his own convenience and for retesting when Work does not comply with the requirements of the Contract Documents.
- Each Special Inspector is responsible to verify and inspect the work for compliance with the approved plans, plan revisions and supporting documents. The Special Inspector shall submit a field report to the Architect/Engineer and to the general contractor for each inspection/test made.
- Axis Structural is not the special inspector for this project. Special inspectors shall meet the qualification requirements set forth in ASTM C1093 for concrete inspection and testing. ASTM D-3740 for soils inspection and testing, ASTM C1093 for masonry inspection and testing. We highly recommend the special inspector for soils be the geotechnical engineer for this project, who is Wellborn Engineering & Surveying.
- The testing lab shall obtain a complete set of project Contract Documents to facilitate monitoring, inspections and testing.
- Special Inspectors shall prepare, sign and submit to the RDPIRC with a copy to the owner and the general contractor (and to the Building Official if he requests) his "Report of Required Special Inspections" after the general contractor completes his work according to the approved plans. The Special Inspector shall prepare his "Report of Required Inspections" using the form approved by and available from the Building Official.
- The general contractor shall provide for the special inspector a "Special Inspector Status Log" at the project site using the standard form approved by and available from the building official so that the special inspectors can sign the form at each of their visits to the site.
- The RDPIRC is responsible to prepare, sign and submit the "Final Report of Required Inspections" for submittal to the Building Official after the general contractor completes his work according to the approved plans. The RDPIRC shall prepare the "Final Report of Required Inspections" using the form approved by and available from the building official.

Earthwork Below Building

Earthwork below the building shall consist of construction of a building pad of select compacted fill material that will classify the soil as not considered expansive (IBC 1808.6.3 and 1808.6.4) according to the geotechnical engineer's report. Earthwork according to the geotechnical engineer's report is summarized below for convenience. Finish floor elevation is assumed at 1165.00 feet - confirm with civil engineering final plans before bidding and construction. Refer to the report for further requirements.

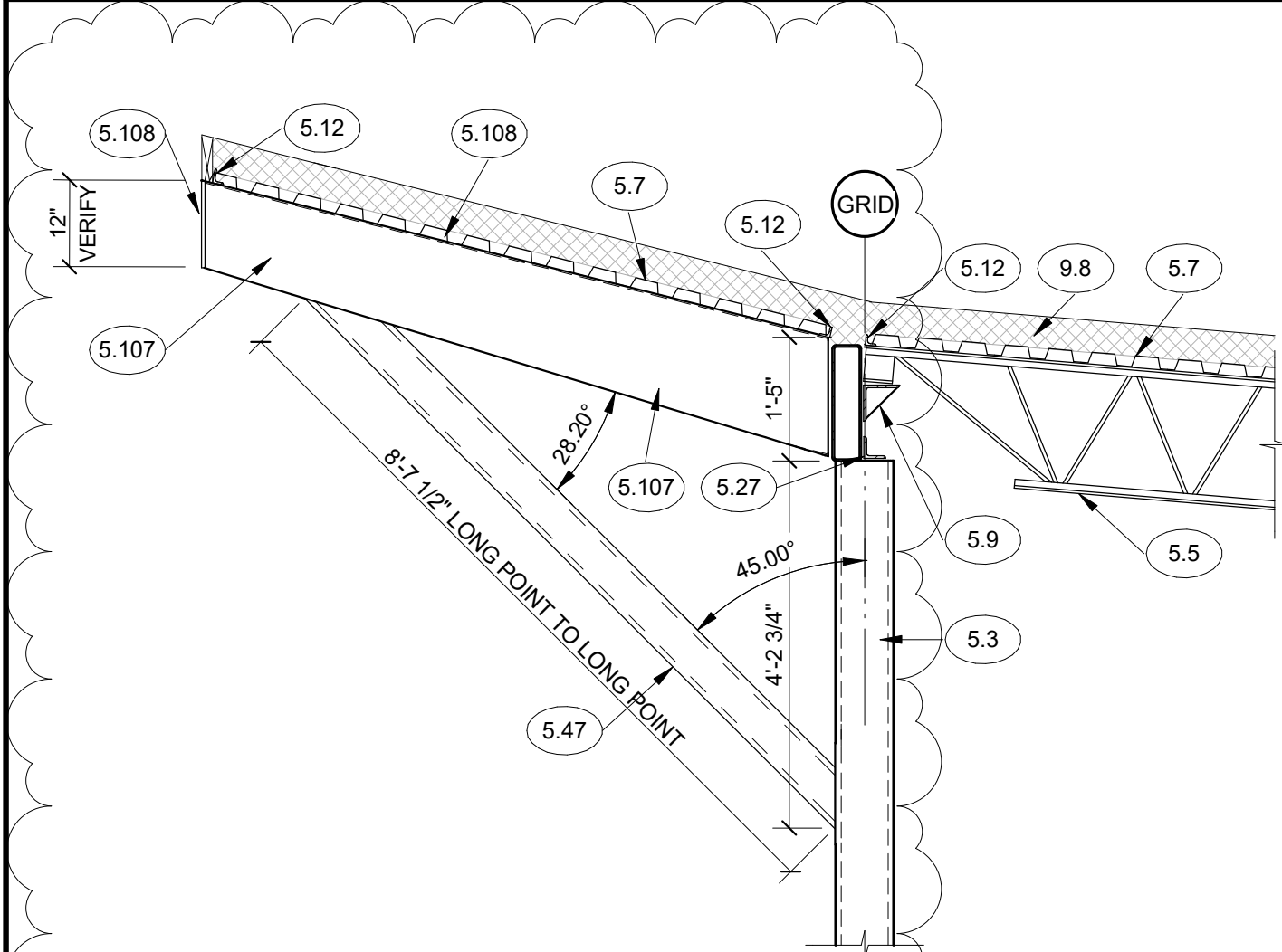
- Excavate to the elevation required for placement of the fill and foundation floor slab. As a minimum, strip topsoil and all organic material from the building foundation area. Excavation shall extend to a cut elevation that removes all fat clays to expose the shallow limestone stratum below or 1161.00, whichever is lower. Heavy rock cutting shall be expected to excavate limestone stratum to a level grade that allows for underground utilities to enter the building area - refer to civil/plumbing plans for elevations of piping running to the building. On sloping sites, subgrade below the required cut elevation should be shaped and benches of select fill. Earthwork shall extend a minimum of 5 feet beyond the building foundation perimeter, drive-thru foundation, and any movement sensitive flatwork. Owner, architect, and/or civil engineer shall determine limits of movement sensitive flatwork.
- Proofroll the subgrade exposed surface as noted in the geotechnical report. Remove all soft or loose soils and replace with select fill.
- General contractor shall coordinate inspection and testing services in order for the special inspector to witness the proofrolling of the exposed subgrade surface and suitability of the underlying soils for foundation support.
- Select fill below the slab shall meet the specifications recommended in the geotechnical report with a maximum PI of 15. Select fill shall be at least 3.5 ft (or 42 inches) minimum thickness compacted in the field not to exceed 8" lifts loose measure to a dry density of at least 95 percent as determined by standard Proctor compaction test method ASTM D-698 at minus two to plus two percent of the optimum moisture content (-2% to +2%).
- General contractor shall coordinate inspection and testing services in order for the special inspector to test moisture/density for each select fill lift at least every 2,500 sf of building pad area for "Density Control of Compaction" according to the latest ASTM D-6938 (at least 3 tests minimum per lift).
- Utility trenches within the building shall be carefully backfilled, moisture conditioned and compacted as recommended in the geotechnical report so that the trench does not become an avenue for moisture to more freely travel under the building. Flowable fill is a structurally acceptable backfill material. General Contractor shall coordinate with the mechanical engineer to determine if special couplings or three-way connections are needed due to the potential vertical rise of the soils.
- General contractor shall coordinate inspection and testing services in order for the special inspector to test moisture/density for each fill lift within the utility trench at least every 100 lf of trench length for "Density Control of Compaction" according to the latest ASTM D-6938 (at least 2 tests minimum per lift per trench).
- The finish surface grading, final drainage of surface water and landscaping shall be constructed in a manner as recommended in the geotechnical report/civil plan/architectural plan.
- General contractor shall coordinate structural engineer's review, the building official inspection, and the special inspector inspection and testing services.
General contractor shall coordinate building official inspection after excavating for beams and placement of all reinforcing steel, with formwork in place.(IBC Chapter 110.3.1) The Building Official may accept a review by the structural engineer in place of the Building Official conducting the review.



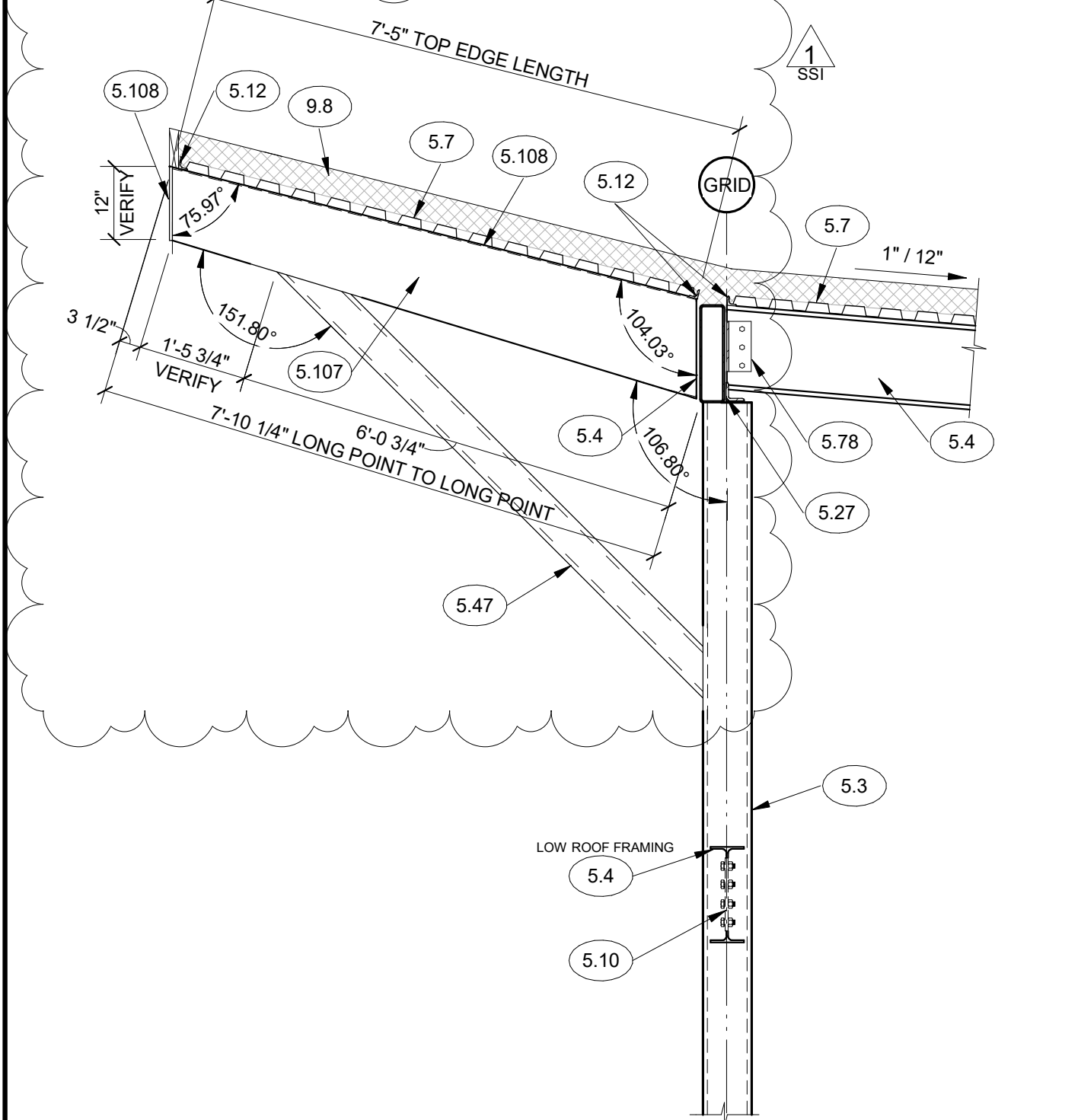
1 Foundation Plan
1/8" = 1'-0"

Date	10/14/26	Description	ADDENDUM 02	Revision	1
1045 CENTRAL PARKWAY N., SUITE 101 DALLAS, TEXAS 75242 PHONE (214) 844-2908 FIRM NO. F-17115 AXISSTRUCTURAL.COM					
CHESNEY MORALES PARTNERS 4901 Broadway, Suite 250 San Antonio, Texas 78209					
FOUNDATION PLAN Security State Bank & Trust Johnson City, Texas					
DRAWN BY: HB & MB PROJECT NO: 26123-0 DATE: 8/12/2026 SHEET NO: S-1.1					

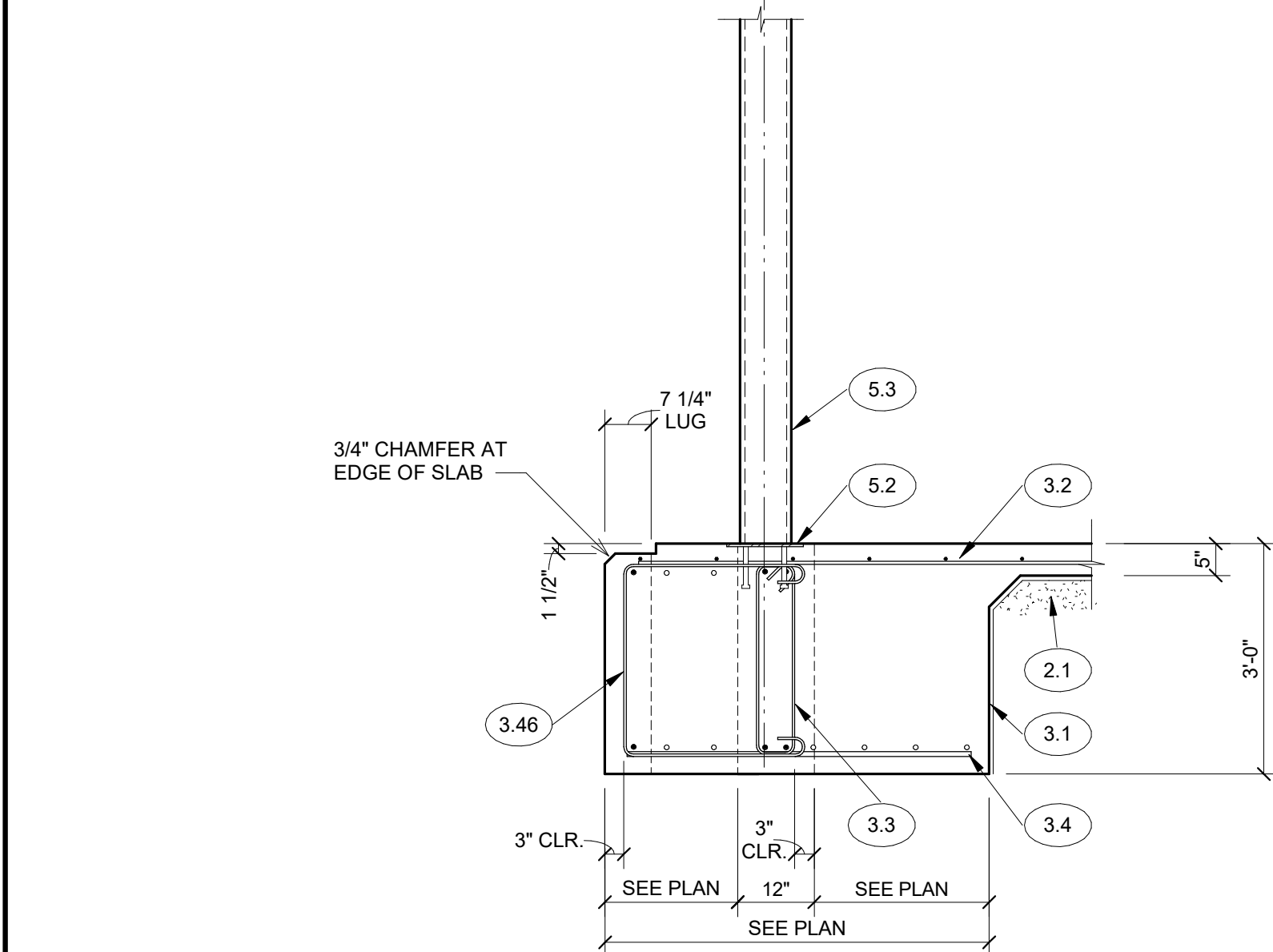
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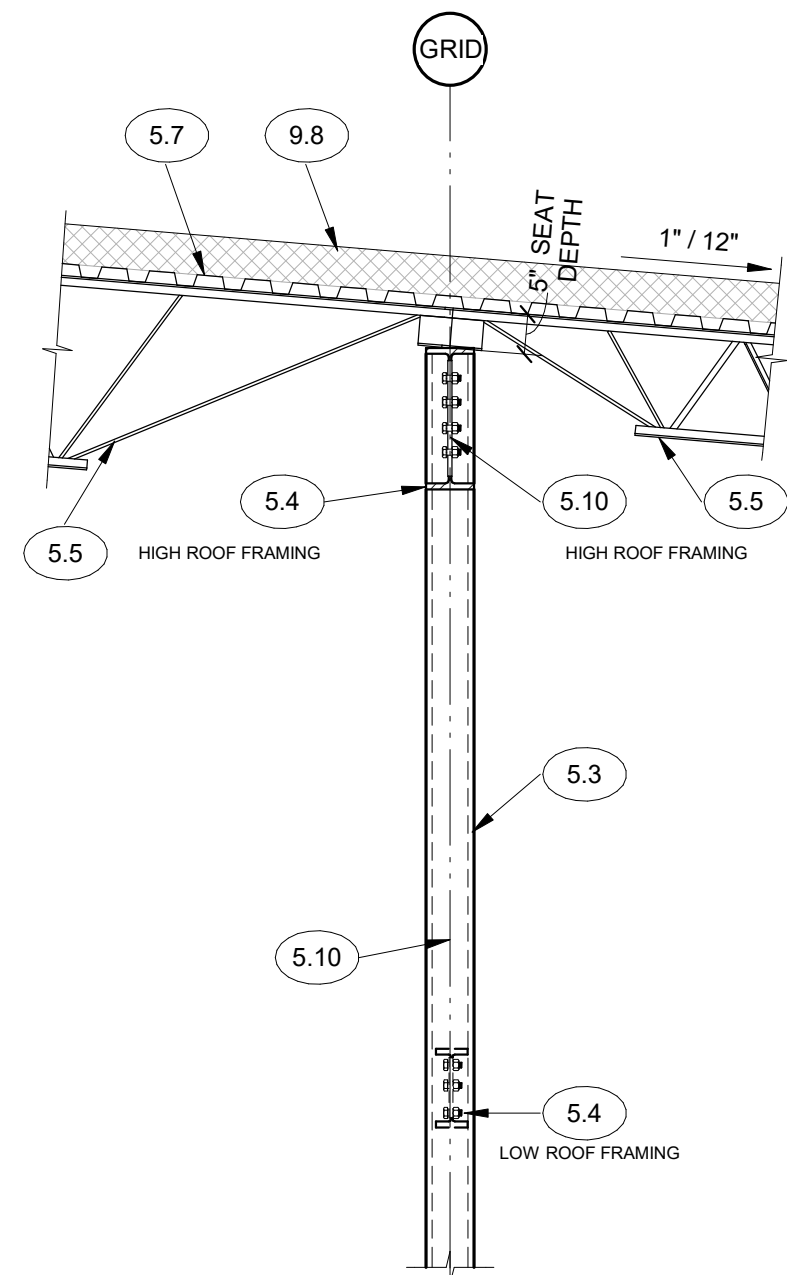
J HIGH-END ROOF BEAMS TO JOIST
1/2" = 1'-0"



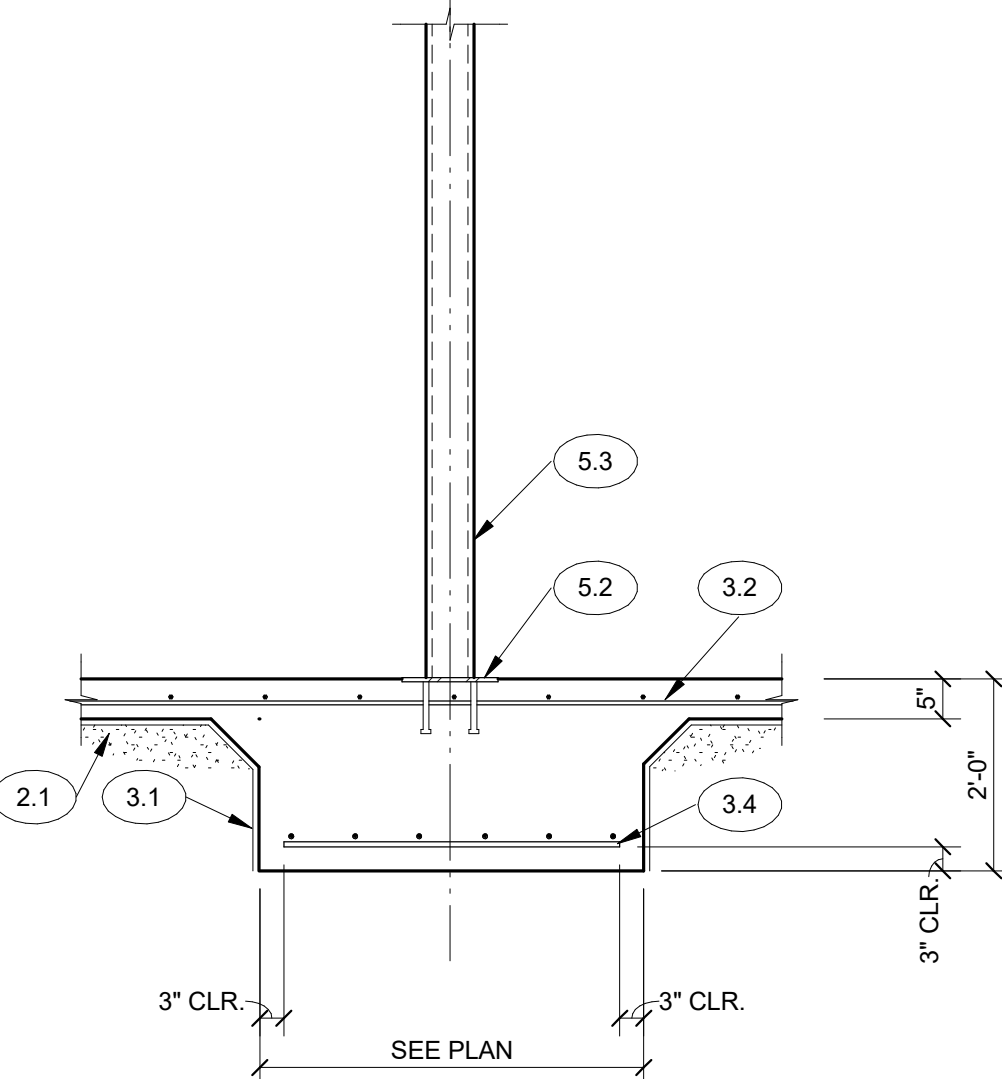
E HIGH-END ROOF BEAMS TO COLUMN
1/2" = 1'-0"



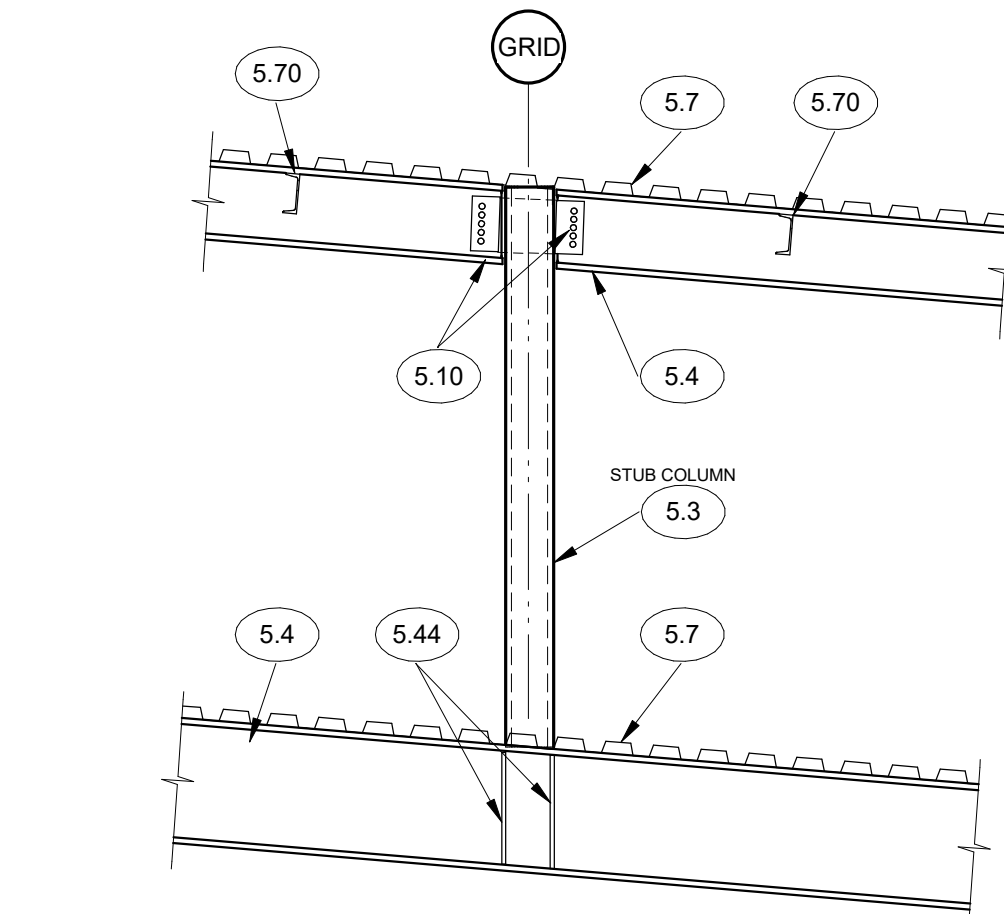
A EXTERIOR FOOTING AT COLUMN
1/2" = 1'-0"



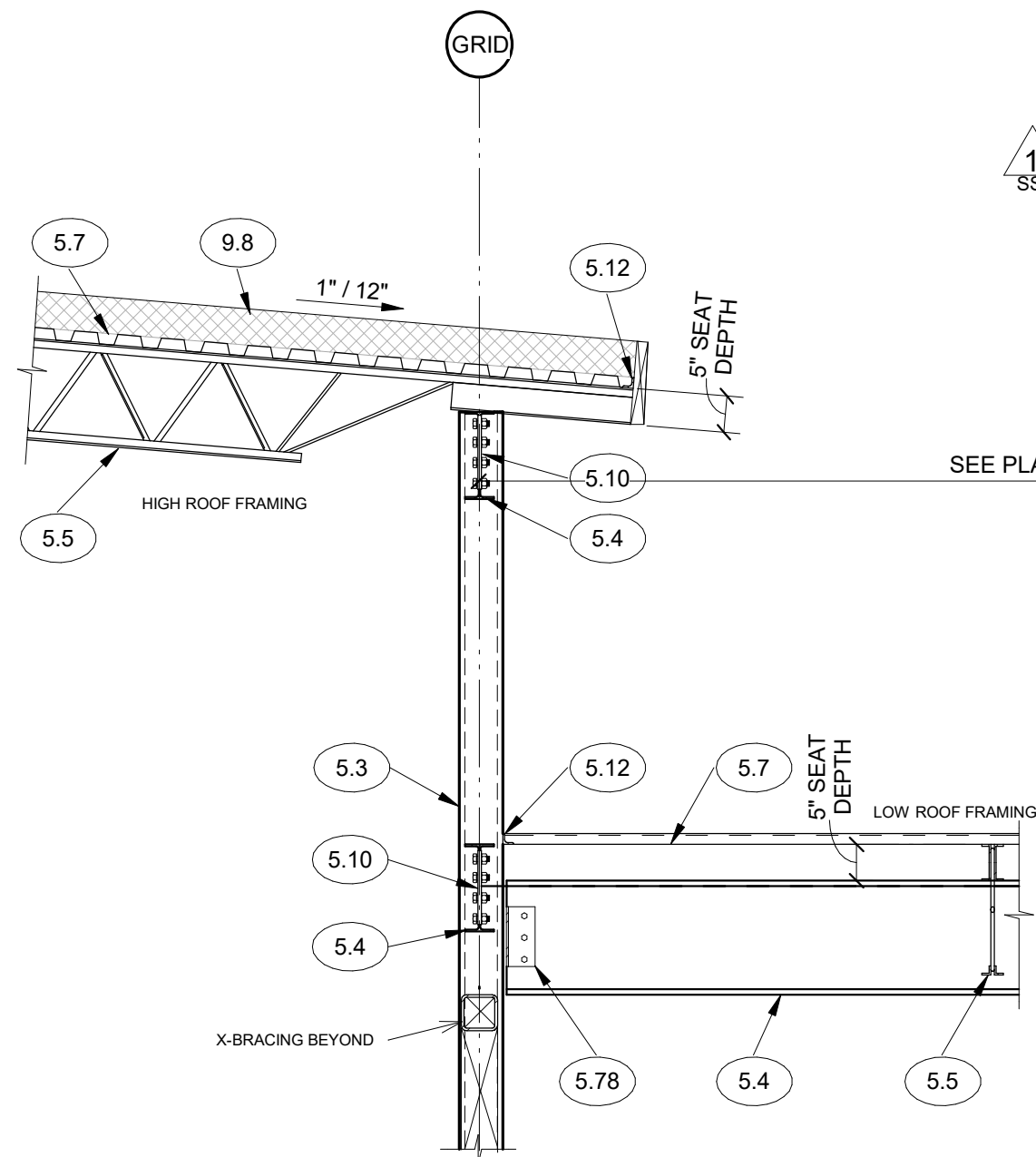
F ROOF BEAM AND JOIST TO COLUMN
1/2" = 1'-0"



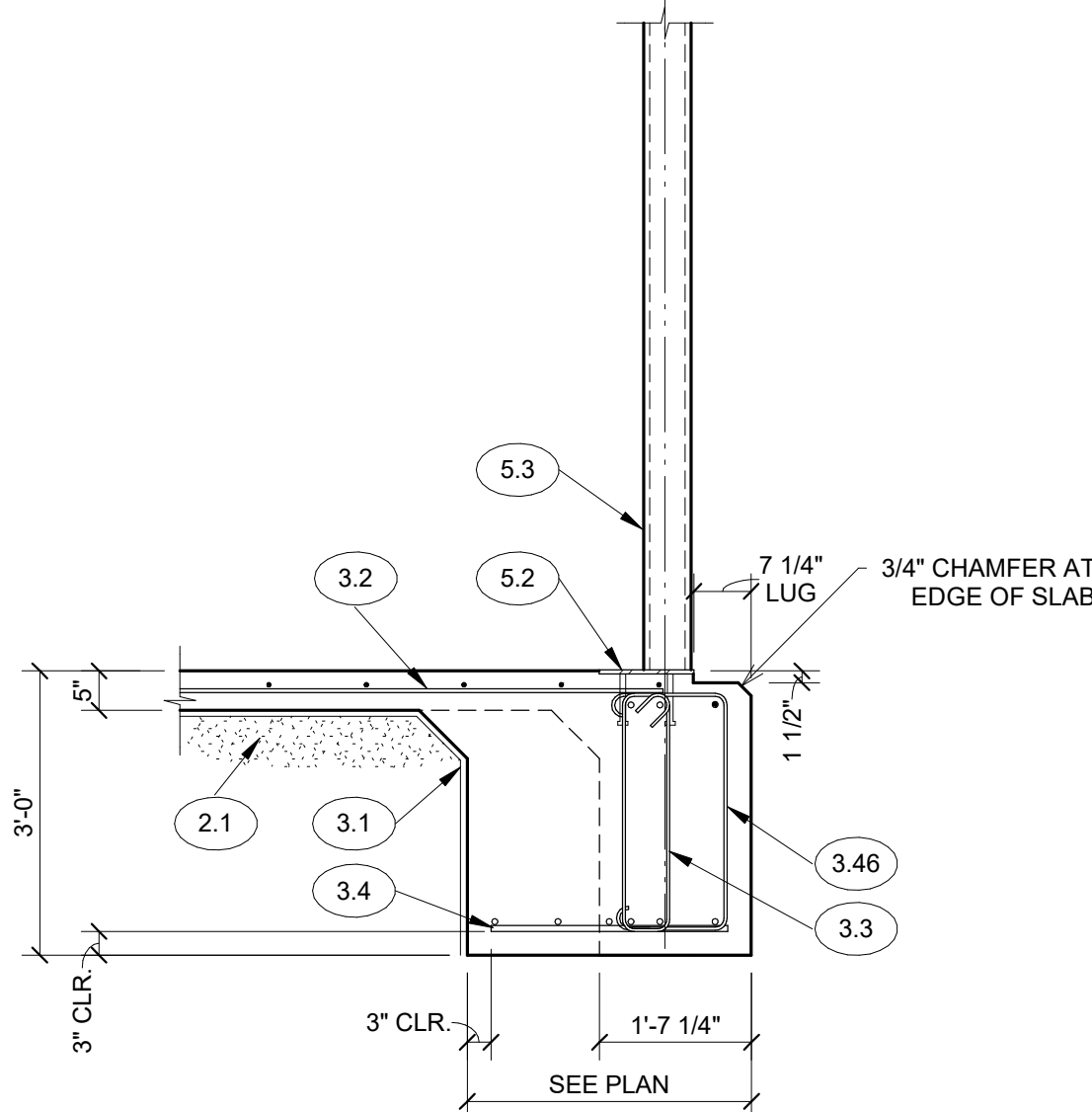
B INTERIOR FOOTING TO FOUNDATION
1/2" = 1'-0"



K LOW ROOF AT STUB COLUMN
1/2" = 1'-0"



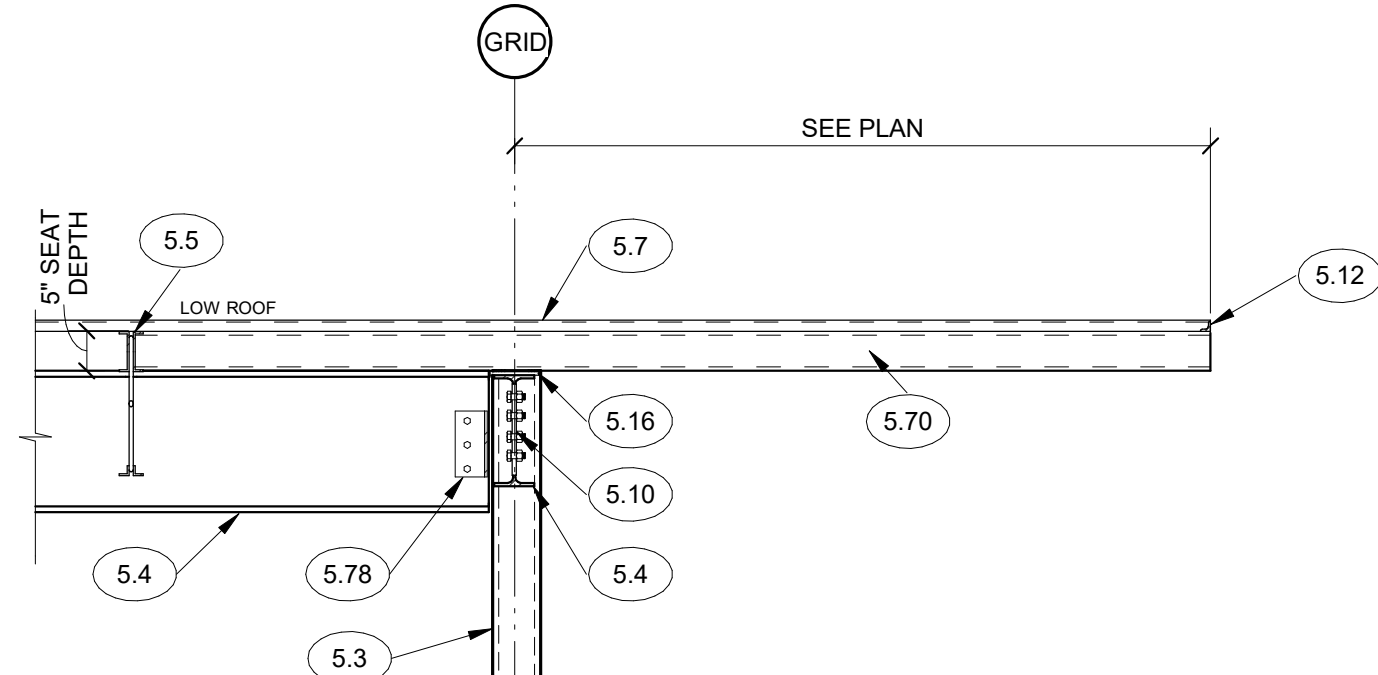
G ROOF TO DRIVE THRU
1/2" = 1'-0"



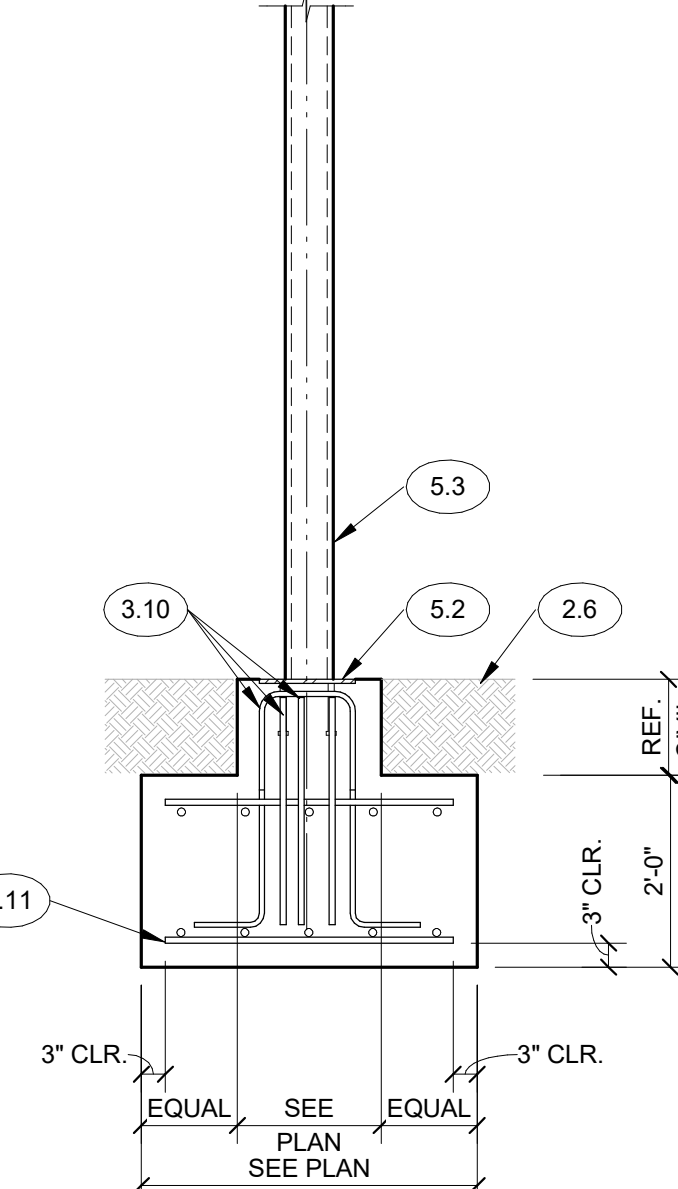
C EXTERIOR FOOTING AT COLUMN
1/2" = 1'-0"

Keynotes

- 2.1 Select structural compacted fill as recommended by the geotechnical engineer.
- 2.6 Flatwork or finished grade - refer to architectural/civil plans.
- 3.1 15 mil thick polyolefin vapor barrier, type recommended to be in contact with the soil or fill under a concrete slab, listed in ASTM 1745 Class A with a permeance less than 0.01 before and after mandatory conditioning test per ASTM E1745 Section 7 (7.1.1-7.1.5) as determined by ASTM E96 or F1249. Polyethylene is not acceptable. Install vapor barrier solidly within and below slab surface with joints lapped at least 6 inches and taped continuously with recommended pressure-sensitive tape. Extend vapor barrier down the sides of the beam trenches and terminate so that it does not extend across the trench bottom. Contractor and Architect (not structural engineer) shall verify with manufacturer's representative that vapor barrier selected is installed in accordance with ASTM E1643-18a. Acceptable manufacturers include Stego, Reef Industries, and Fortifiber only with no alternative submittals permitted.
- 3.2 #3 at 22 inches on center each way centered in concrete slab thickness. Extend slab reinforcing to 1 1/2" from slab edge. Start slab steel spacing not more than 6 inches from top inside beam bar and not more than 1 1/2 inches from slab edges along leave-outs or trenches. Provide 3-#4 diagonal bars x 5' long above typical slab reinforcing at all slab interior corners. Install bars around all slab leave-out edges where main slab reinforcing bars are cut creating a discontinuous line of steel.
- 3.3 12" wide grade beam reinforcing shall be: 2- #6 continuous bars top and bottom with #3 closed ties at 18 inches on center. 1'-6" to 2'-0" wide grade beam reinforcing shall be: 3- #6 continuous bars top and bottom with #3 closed ties at 18 inches on center. Grade beams wider than 2'-0" shall be reinforced with: #6 continuous bars top and bottom at 8" on center with #3 closed ties at 18 inches on center. Start stirrup spacing at ends of horizontal beam bars. Lap #6 "Z" bars to horizontal bars where beam steps down greater than 3". Lap 3- #6 corner bars top and 3- #6 corner bars bottom to horizontal beam bars at all beam corners and dead end beam intersections. For beams with depth exceeding 3'-0", add #4 continuous mid-height horizontal bars at each beam face at 12" on center.
- 3.4 Footing reinforcing shall be #6 bars at 8" on center each way 3" clear from the sides, footing bottom and below the beam reinforcing. Continue beam reinforcing through the footing.
- 3.10 Concrete column pedestal reinforced with 6-#6 vertical bars with 90 degree bends tied to bottom footing reinforcing. Extend ends of bars to 1 1/2" from top of pedestal. At the top of pedestal, provide #3 closed ties at 6" on center (3 at 3" on center top) spaced vertically down the pedestal starting at 1 1/2" from top of pedestal down to no less than 1 1/2" from the top of the footing. Across the top of the pedestal, provide two (2) #4 U-bars in each direction spaced 6" off outer edge of pedestal cage. Legs of U-bars shall be 20" long on each end.
- 3.11 Footing reinforcing shall be #6 bars at 8" on center both ways 3" clear from bottom and top of footing.
- 3.46 2- #6 continuous top and bottom with #3 wider stirrups at 12" on center at widened grade beam lug locations.
- 5.2 12" x 12" x 3/4" column base plate with 4- 3/4" diameter HCA x 8" long. Center of HCA anchors shall be located 2" off the edge of the plate in each direction unless noted otherwise.
- 5.3 Steel tube column - see plan.
- 5.4 Steel beam - see plan.
- 5.5 Steel joist - see plan.
- 5.7 Metal roof deck as noted/specified.
- 5.9 4" x 6" x 3/8" x (beam flange width +1") angle seat (LLV) with 3/8" thick stiffener plate. Weld to column/panel embed continuously and bolt to beam bottom flange with 2- 3/4" diameter HS bolts.
- 5.10 3/8" thick gusset plate through column welded continuous. Bolt beam with number of 3/4" diameter high strength bolts as required for 1/2 shear in AISC beam tables.
- 5.12 1 1/4" x 1 1/4" x 1/4" angle (LLH) continuous at roof deck edge. Weld to each joist or supporting structural member with 2" welds. At building corners bolt to wall with 5/8" diameter wedge girth anchors. OR weld to embedded plate in panel. Weld angle ends together with 2" x 2" x 1/4" lap angle x 12" long OR splice plate x 12" long on each leg.
- 5.16 1/4" thick HSS column cap plate.
- 5.27 3" x 3" x 1/4" angle welded to top column and beam at eave location.
- 5.44 3/8" thick vertical stiffener plate at column edges/centerline.
- 5.47 HSS 6" x 6" x 1/4" thick diagonal brace continuously welded to column and tapered roof beam at each tapered outrigger beam location.
- 5.70 C5 x 6.7 outriggers. See plan for location.
- 5.78 L5x3x3/8 steel angle x beam web depth with the short leg welded to the column/panel embed/beam web and bolted to the supported beam with number of 3/4" diameter high strength bolts as required for 1/2 shear in AISC beam tables.
- 5.107 HSS tapered beam as noted on the plan. Create this tapered beam out of HSS18x6x1/4 steel tube with the remaining section cut out of the noted tube with steel flat plate welded along all exposed edges as keyed to enclose the tube along the top and front ends. Refer to the extruded detail for this outline.
- 5.108 6" wide x 1/4" thick steel plate welded along all edges and ground down per the noted AESS specifications.
- 9.8 Roofing - refer to architectural.



H ROOF TO DRIVE THRU- CONT.
1/2" = 1'-0"



D FOUNDATION AT DRIVE-THRU
1/2" = 1'-0"

Date	Description	Revision
10/14/26	ADDENDUM 02	1

STATE OF TEXAS
MIKE BRATTEN
121512
LICENSED PROFESSIONAL ENGINEER
10/14/26
Copy No. 00000
Axis Structural, LLC.

1045 CENTRAL PARKWAY N., SUITE 101
SAN ANTONIO, TEXAS 78232
PHONE (210) 844-2908
FIRM NO. F-17115
AXISSTRUCTURAL.COM

AXIS
STRUCTURAL

CHESNEY MORALES PARTNERS
4901 Broadway, Suite 250
San Antonio, Texas 78209

TYPICAL STRUCTURAL SECTIONS AND DETAILS

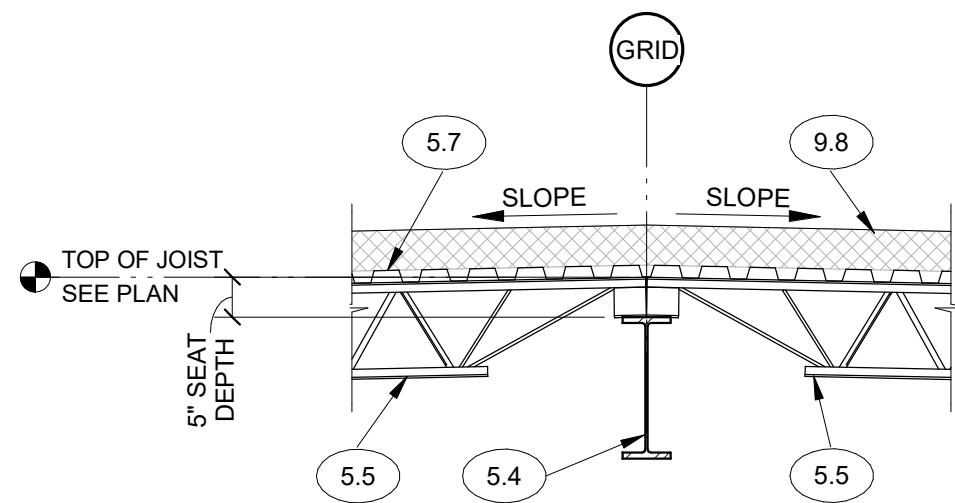
Security State Bank & Trust
Johnson City, Texas

DRAWN BY: HB & MB
PROJECT NO: 26123-0
DATE: 8/12/2026
SHEET NO: **S-2.1**

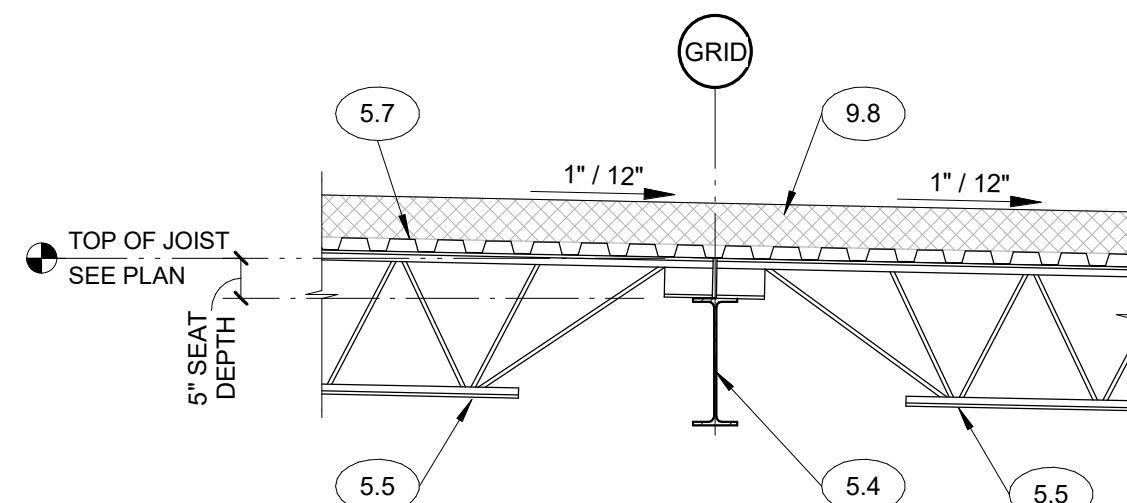
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Keynotes

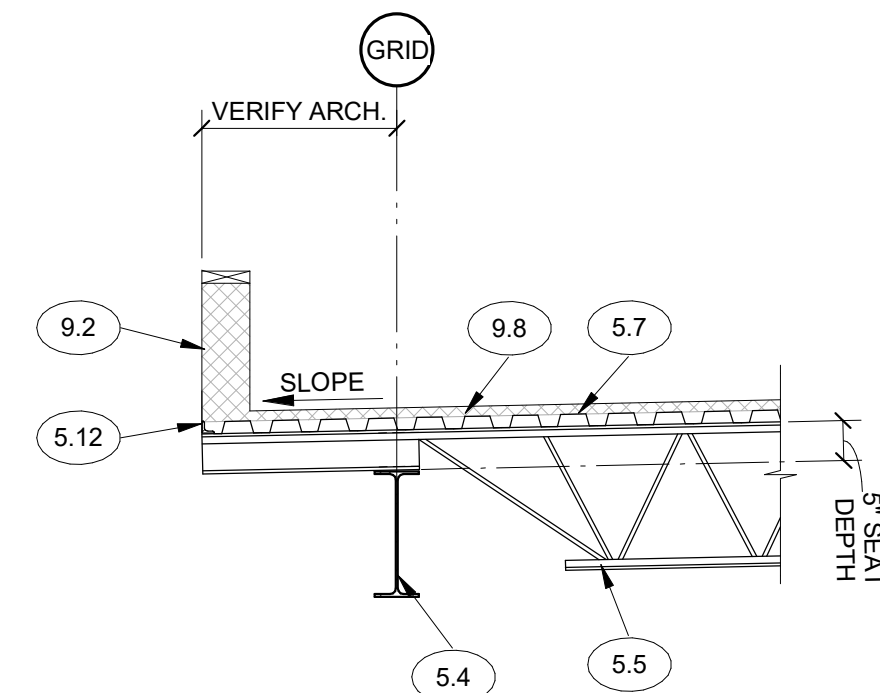
- 2.1 Select structural compacted fill as recommended by the geotechnical engineer.
- 3.1 15 mil thick polyolefin vapor barrier, type recommended to be in contact with the soil or fill under a concrete slab, listed in ASTM 1745 Class A with a permeance less than 0.01 before and after mandatory conditioning test per ASTM E1745 Section 7 (7.1, 1-7.1.5) as determined by ASTM E96 or F1249. Polyethylene is not acceptable. Install vapor barrier solidly within and below slab surface with joints lapped at least 6 inches and taped continuously with recommended pressure-sensitive tape. Extend vapor barrier down the sides of the beam trenches and terminate so that it does not extend across the trench bottom. Contractor and Architect (not structural engineer) shall verify with manufacturer's representative that vapor barrier selected is installed in accordance with ASTM E1643-18a. Acceptable manufacturers include Stego, Reef Industries, and Fortifiber only with no alternative submittals permitted.
- 3.2 #3 at 22 inches on center each way centered in concrete slab thickness. Extend slab reinforcing to 1 1/2' from slab edge. Start slab steel spacing not more than 6 inches from top inside beam bar and not more than 1 1/2 inches from slab edges along leave-outs or trenches. Provide 3-#4 diagonal bars x 5' long above typical slab reinforcing at all slab interior corners. Install bars around all slab leave-out edges where main slab reinforcing bars are cut creating a discontinuous line of steel.
- 3.3 12" wide grade beam reinforcing shall be: 2- #6 continuous bars top and bottom with #3 closed ties at 18 inches on center. Grade beams wider than 2'-0" shall be reinforced with: #6 continuous bars top and bottom at 8" on center with #3 closed ties at 18 inches on center. Start stirrup spacing at ends of horizontal beam bars. Lap #6 "Z" bars to horizontal beam bars where beam steps down greater than 3". Lap 3- #6 corner bars top and 3- #6 corner bars bottom to horizontal beam bars at all beam corners and dead end beam intersections. For beams with depth exceeding 3'-0", add #4 continuous mid-height horizontal bars at each beam face at 12" on center.
- 3.4 Footing reinforcing shall be #6 bars at 8" on center each way 3" clear from the sides, footing bottom and below the beam reinforcing. Continue beam reinforcing through the footing.
- 3.5 3/16" wide saw cut joints. Joints shall be cut 4 to 8 hours after concrete is set and shall be a minimum of at least 1/4 of the slab thickness in depth. Reinforcement shall be continuous through sawed joints. Fill saw cuts with EUOCO 700 Semi-Rigid industrial floor joint filler as manufactured by the Euclid Chemical Company. Follow manufacturers recommendations and directions for application of product.
- 3.13 3 mats of #5 bars at 4" on center. Stagger/offset mat in both directions.
- 3.62 4" thick concrete slab on non-composite-metal floor deck as noted.
- 5.2 12" x 12" x 3/4" column base plate with 4- 3/4" diameter HCA x 8" long. Center of HCA anchors shall be located 2" off the edge of the plate in each direction unless noted otherwise.
- 5.3 Steel tube column - see plan.
- 5.4 Steel beam - see plan.
- 5.5 Steel joist - see plan.
- 5.6 Metal floor deck as noted/specified.
- 5.7 Metal roof deck as noted/specified.
- 5.10 3/8" thick gusset plate through column welded continuous. Bolt beam with number of 3/4" diameter high strength bolts as required for 1/2 shear in AISC beam tables.
- 5.12 1 1/4" x 1 1/4" x 1/4" angle (LLH) continuous at roof deck edge. Weld to each joist or supporting structural member with 2" welds. At building corners bolt to wall with 5/8" diameter wedge anchors. OR weld to embedded plate in panel. Weld angle ends together with 2" x 2" x 1/4" lap angle x 12" long OR splice plate x 12" long on each leg.
- 5.44 3/8" thick vertical stiffener plate at column edges/centerline.
- 5.61 C5 x 6.7 at 5'-0" on center at all canopy locations. See plan.
- 5.63 MC12 X 10.6 outer perimeter fascia channel at all canopy locations - see plan.
- 5.70 C5 x 6.7 outriggers. See plan for location.
- 5.77 HSS6x2x1/4 steel tube laid flat running continuous along top of beam at low mechanical mezzanine slab area and low roof framing interface. Weld ends of tube to steel columns at each end and support tube below joists seats with HSS 4x4x1/4" stub column.
- 5.78 L5x3x3/8 steel angle x beam web depth with the short leg welded to the column/panel embed/beam web and bolted to the supported beam with number of 3/4" diameter high strength bolts as required for 1/2 shear in AISC beam tables.
- 5.98 At mechanical mezzanine, install a 4" x 3" x 1/4" steel angle (LLV) around floor perimeter.
- 5.200 Light-gage metal stud as noted.
- 9.2 Exterior finish - refer to architectural.
- 9.8 Roofing - refer to architectural.
- 9.9 Freezer wall panel - refer to architectural.



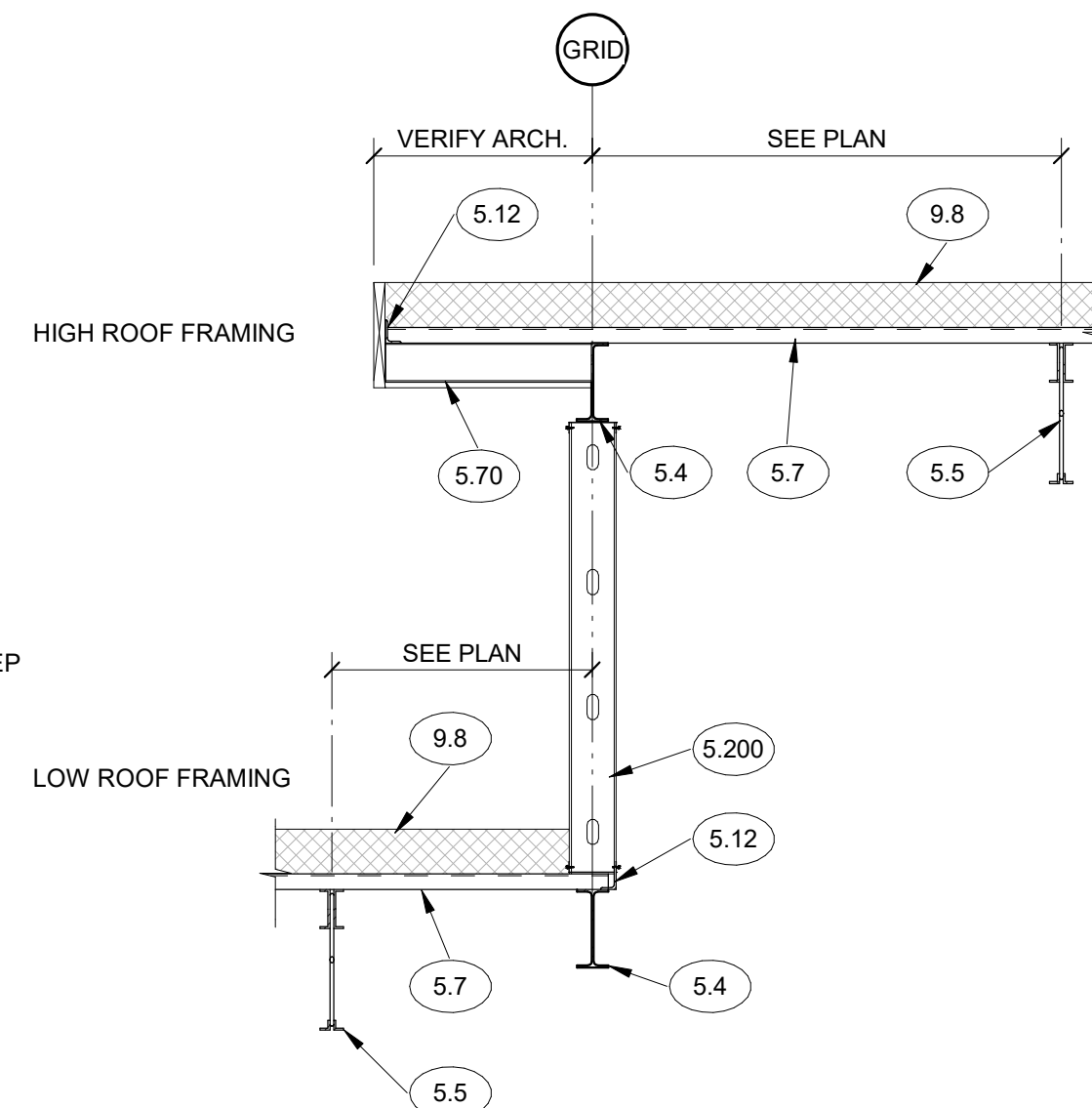
6 JOISTS AT DRIVE THRU ROOF
1/2" = 1'-0"



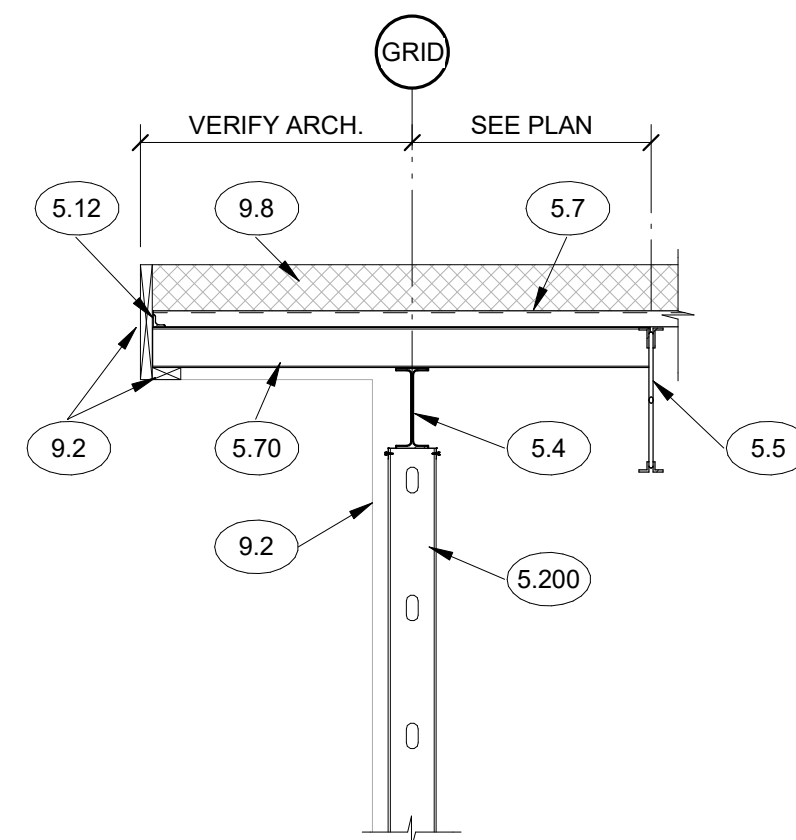
7 JOISTS AT INTERIOR BEAM
1/2" = 1'-0"



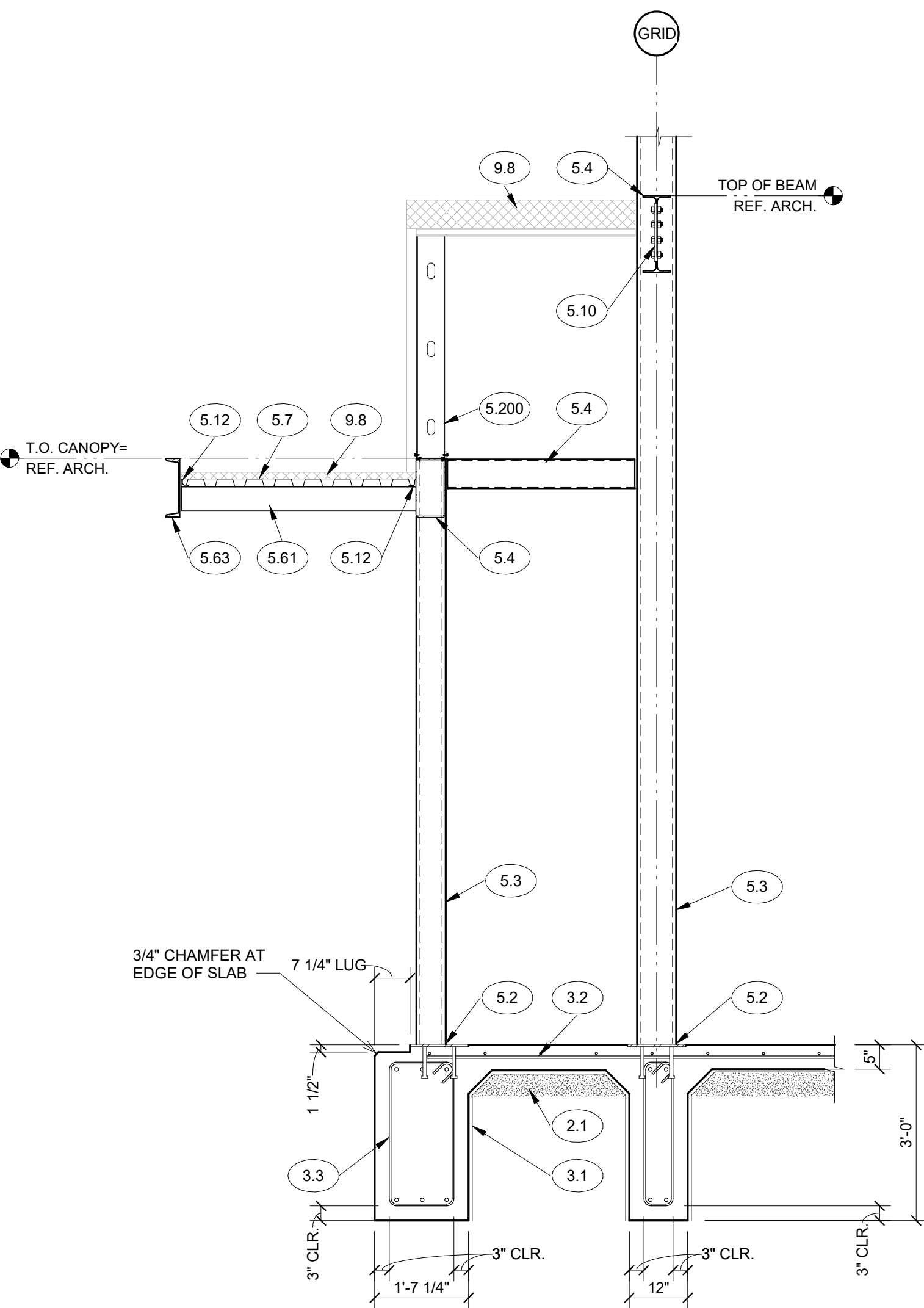
8 OUTRIGGERS TO BEAM AND JOIST
1/2" = 1'-0"



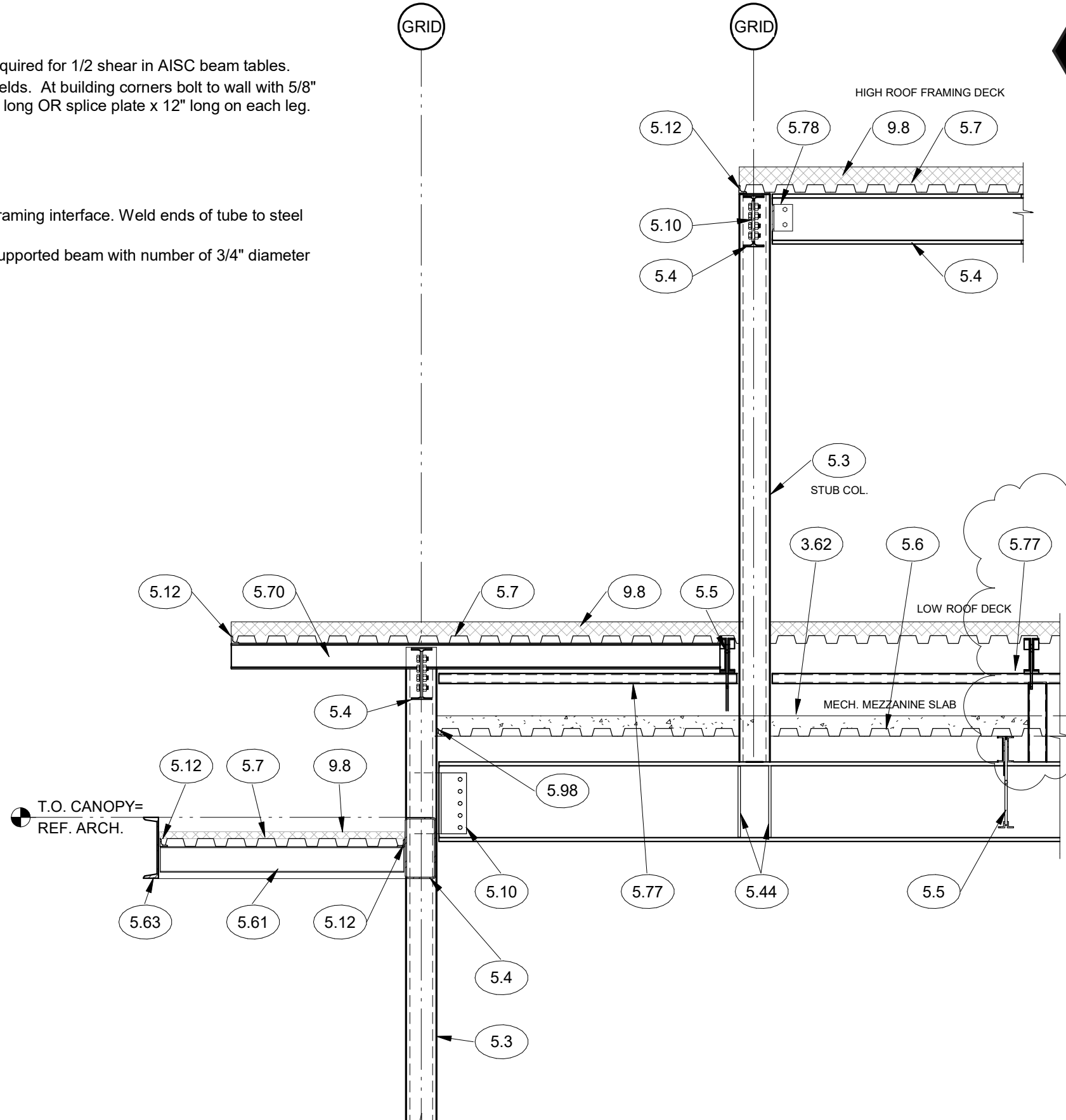
2 LOW ROOF TO HIGH ROOF - END WALL
1/2" = 1'-0"



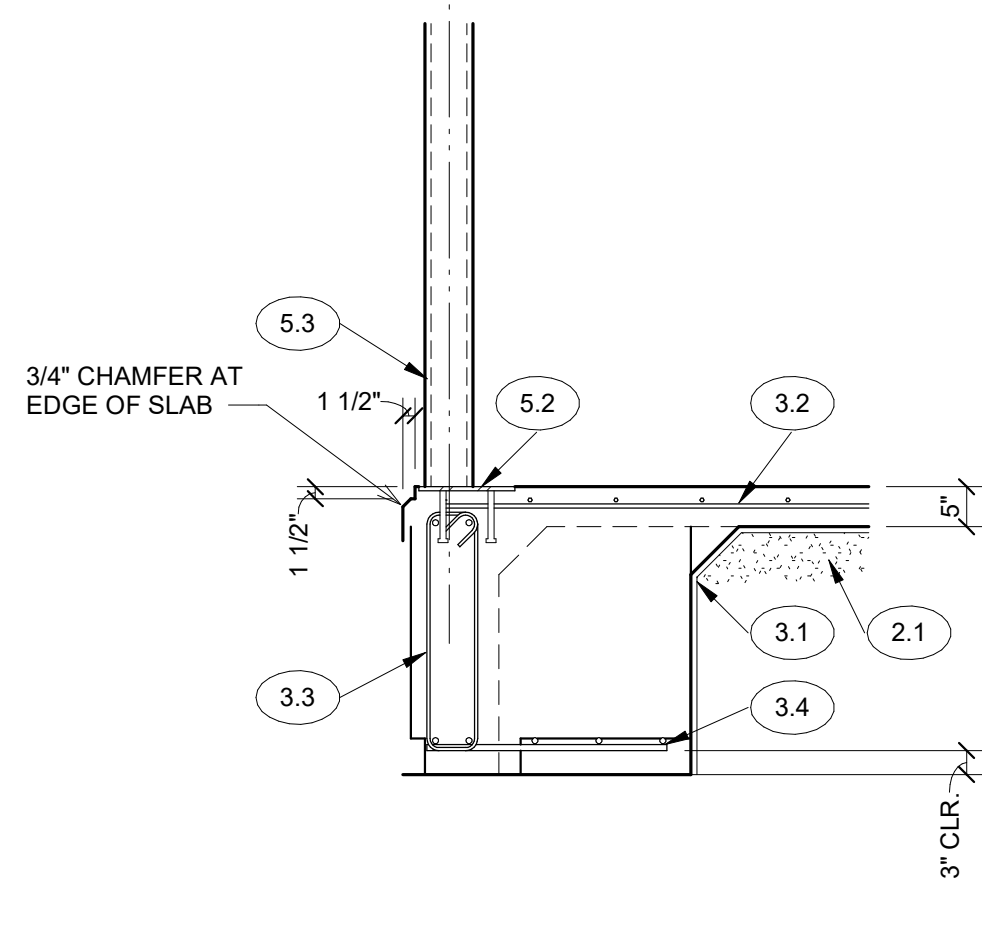
1 EXTERIOR WALL TO LOW ROOF - END WALL
1/2" = 1'-0"



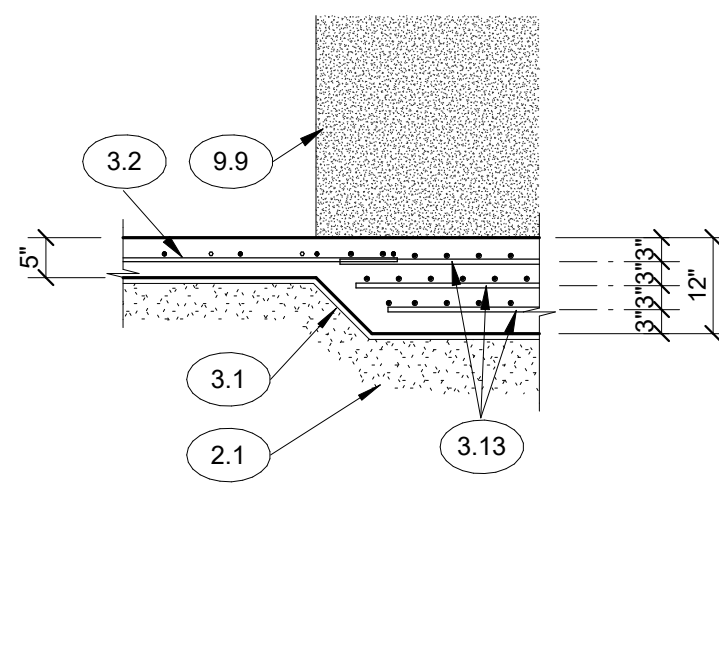
A VESTIBULE AT FOUNDATION
1/2" = 1'-0"



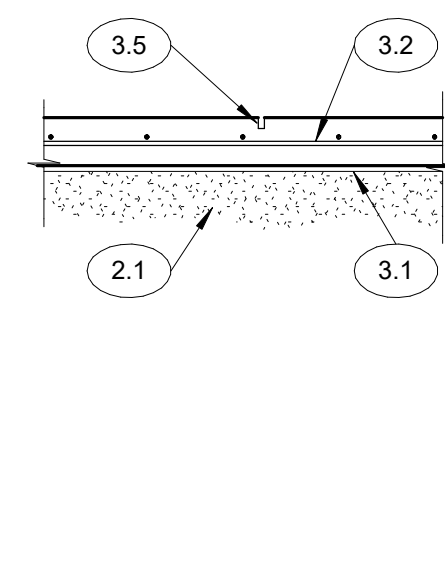
F ROOF TO EXTERIOR COLUMN AT CANOPY
1/2" = 1'-0"



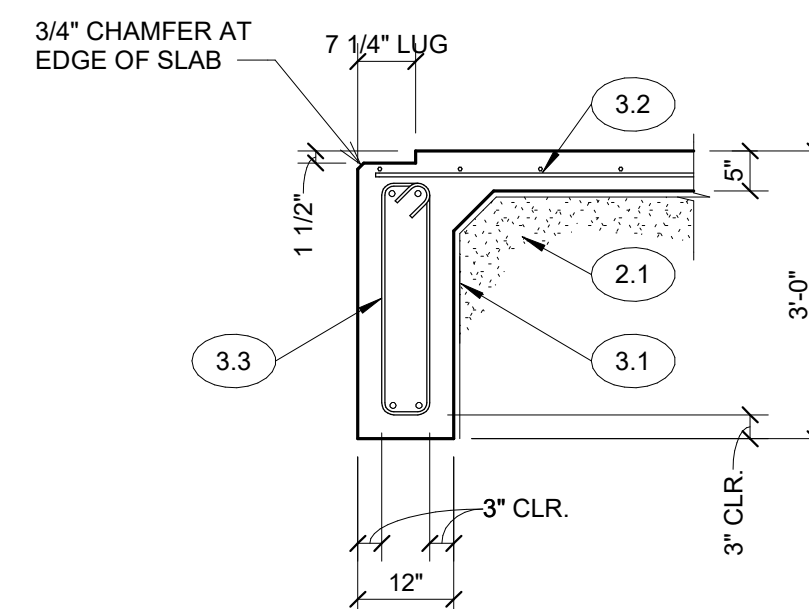
C EXTERIOR FOOTING - NO LUG
1/2" = 1'-0"



D FOUNDATION AT VAULT
1/2" = 1'-0"



E SAWCUT JOINT
1/2" = 1'-0"



B EXTERIOR GRADE BEAM - LUG
1/2" = 1'-0"

Date	10/14/26
Description	APPENDUM 02
Revision	1
1045 CENTRAL PARKWAY N., SUITE 101 SAN ANTONIO, TEXAS 78202 PHONE (210) 844-2908 FIRM NO. F-17115 AXISSTRUCTURAL.COM	
CHESNEY MORALES PARTNERS 4901 Broadway, Suite 250 San Antonio, Texas 78209	
TYPICAL STRUCTURAL SECTIONS AND DETAILS Security State Bank & Trust Johnson City, Texas	
DRAWN BY:	HB & MB
PROJECT NO:	26123-0
DATE:	8/12/2026
SHEET NO:	S-2.2

LIGHTING CONTROL MATRIX										
ID	ROOM TYPE	LOCAL CONTROL FUNCTIONS	SENSOR MOUNTING	LIGHTING AUTO ON LEVEL	OCCUPANCY SHUTOFF	TIMECLOCK CONTROL	DIMMABLE	DAYLIGHT CONTROL	RECEPTACLE CONTROL	NOTES
A	OFFICE (ENCLOSED)	ON/OFF/DIM	WALL (INTEGRAL)	0%	20 MIN	NO	YES	NO	NO	1.
B	IT / ELEC ROOM	ON/OFF	N/A		N/A	NO	NO	NO	NO	
C	RESTROOM	ON/OFF	CEILING	100%	20 MIN	NO	NO	NO	NO	
D	WORKROOM	ON/OFF/DIM	CEILING	0%	20 MIN	NO	YES	NO	NO	
E	STORAGE	ON/OFF	WALL (INTEGRAL)	0%	20 MIN	NO	NO	NO	NO	
F	CORRIDOR	ON/OFF	CEILING	100%	20 MIN	NO	NO	NO	NO	
G	BREAK ROOM	ON/OFF/DIM	CEILING	0%	20 MIN	YES	YES	NO	NO	
H	MEETING ROOM	ON/OFF/DIM	CEILING	0%	20 MIN	NO	YES	NO	NO	
J	BANK LOBBY / TELLER	ON/OFF/DIM	CEILING	N/A	N/A	YES	YES	NO	NO	
K	VAULT / VIEWING ROOM	ON/OFF	WALL (INTEGRAL)	100%	20 MIN	NO	NO	NO	NO	
L	OFFICE (ENCLOSED)	ON/OFF/DIM	CEILING	0%	20 MIN	NO	YES	NO	NO	
X1	EXTERIOR GENERAL	N/A	INTEGRAL	100%	15 MIN	YES	YES	YES	NO	2.
X2	EXTERIOR DRIVE-THRU	N/A	INTEGRAL	100%	15 MIN	YES	YES	YES	NO	2.
SCHEDULE GENERAL NOTES: A. PROVIDE ALL POWER PACKS, RELAYS, PHOTOCELLS, TIME CLOCKS, CONTROLLERS, AND BRIDGING DEVICES AS REQUIRED FOR A FUNCTION CONTROL SYSTEM. B. OCCUPANCY SENSORS TO BE PLACED PER MANUFACTURER RECOMMENDATIONS. C. CONTRACTOR TO PROVIDE SHOP DRAWING INDICATING LOCATIONS OF ALL CONTROL DEVICES AS PART OF THE LIGHTING SUBMITTAL FOR APPROVAL PRIOR TO INSTALLATION. D. AUTO ON LEVEL : 0% = MANUAL ON. E. REFER TO LIGHTING PLANS FOR ROOM TYPE DESIGNATIONS PER THE ID COLUMN ABOVE. F. ALL EMERGENCY LIGHTING CONTROLLED BY AUTOMATIC MEANS SHALL BE CONTROL THROUGH A LISTED UL924 CONTROL DEVICES THAT BRINGS LIGHTS TO FULL OUTPUT UPON LOSS OF NORMAL BRANCH POWER, SIGNAL FROM FIRE ALARM SYSTEM, OR SIGNAL FROM OTHER DESIGNATED CONTROL PANEL. G. DAYLIGHTING CONTROL IN DAYLIGHTING AREAS ONLY. SEE CODED NOTE ON PLAN FOR LOCATIONS. H. MANUAL OFF: WHEN LIGHTS ARE MANUALLY TURNED OFF, THEY ARE TO REMAIN OFF UNTIL THEY ARE MANUALLY TURNED ON, THEN AUTO-ON/OFF CONTROL WILL TAKE OVER. I. ALL ROOMS TO BE TIED TO THE LOW VOLTAGE CONTROL SYSTEM UNLESS NOTED OTHERWISE. J. ALL WALL MOUNTED OCCUPANCY SENSORS SHALL HAVE INTEGRAL ON/OFF BUTTONS, AND UP/DOWN DIMMING BUTTONS WHERE APPLICABLE. K. ALL ROOMS WITH AUTOMATIC DAYLIGHT CONTROL SHALL ADJUST ONCE THE LIGHT LEVELS REACH 50% AT 36" AFF. L. LIGHTING CONTROL ZONES: LIGHTS TO BE CONTROLLED TOGETHER, PER LOWER-CASE LETTER. M. WHERE INDICATED AS "DIMMABLE" THE PROVIDED CONTROLS SHALL BE COMPATIBLE WITH THE FIXTURE SHOWN IN THE LUMINAIRE SCHEDULE. SCHEDULE KEYNOTES: 1. AUTOMATIC SHUTOFF WOULD ENDANGER OCCUPANT SAFETY OR SECURITY, THEREFORE IS EXEMPT FROM OCCUPANT SENSOR CONTROLS AND TIME-SWITCH CONTROLS PER IECC C405 2.2. 2. AUTOMATIC SHUTOFF PROVIDED VIA TIMECLOCK. COORDINATE WITH OWNER FOR TIMECLOCK SCHEDULE.										

ELECTRICAL EQUIPMENT SCHEDULE								
ID	VOLTAGE	PHASE	DISC. AMP RATING	DISCONNECT TYPE	FUSING	ENCLOSURE RATING	FEEDER OR BRANCH WIRE SIZE	NOTES
CONDENSING UNIT								
CU-1	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #10, 1#10G, 3/4"C	
CU-2	208 V	3	60 A	HEAVY-DUTY	NON-FUSED	NEMA 1	3 #8, 1 #10G, 1"C	
CU-3	208 V	3	60 A	HEAVY-DUTY	NON-FUSED	NEMA 1	3 #8, 1 #10G, 1"C	
CU-4	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
CU-5	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
EXHAUST FAN								
EF-1	120 V	1	20 A	N/A	N/A	NEMA 1	2 #12, 1 #12G, 3/4"C	
FAN COIL UNIT								
FC-1	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
FC-2A	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
FC-2B	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
FC-3	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
FC-4	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	
FC-5	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #12, 1 #12G, 3/4"C	1
WATER HEATER								
EW-1	208 V	1	30 A	HEAVY-DUTY	NON-FUSED	NEMA 1	2 #10, 1#10G, 3/4"C	
SCHEDULE GENERAL NOTES: A. PROVIDE ALL DISCONNECTS WITH LOCK OUT TAG OUT PROVISIONS B. MOUNT DISCONNECTS TO STRUCTURE ADJACENT TO EQUIPMENT. DO NOT MOUNT TO EQUIPMENT. C. REFER TO GENERAL NOTES AND SPECIFICATIONS FOR ADDITIONAL INFORMATION. D. EC SHALL INSTALL ALL DISCONNECTS, STARTERS, VFDs, & MAKE ELECTRICAL TERMINATIONS. HVAC CONTROL WIRING & TERMINATIONS BY CONTROLS CONTRACTOR. SCHEDULE NOTES: 1. FED FROM OUTDOOR UNIT.								

LIGHTING FIXTURE SCHEDULE									
TYPE	CONSTRUCTION DESCRIPTION	MFR	PRODUCT		LUMENS	ELECTRICAL			NOTES
			MODEL			VOLT	INPUT WATTS		
D2	3" DOWNLIGHT	LEDRA BRANDS	NU3-RD-SW-10LM-30K-80-CQR-D40		1000	120 V	9 W		
D2E	3" DOWNLIGHT TO MATCH TYPE D2, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK	LEDRA BRANDS	NU3-RD-SW-10LM-30K-80-CQR-D40-EM7		1000	120 V	9 W		
DTL	DRIVE THRU LIGHTING		OWNER PROVIDED FIXTURE			120 V	0 W		3.
L1	4" STRIP	LITHONIA LIGHTING	FMLWL 48 835 ZT		4587	120 V	41 W		
L1E	4" STRIP TO MATCH TYPE L1, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK.	LITHONIA LIGHTING	FMLWL 48 835 ZT		4587	120 V	41 W		
L2	2X4 RECESSED	LITHONIA LIGHTING	ENVX 2X4 HRGC 4800LM 80CRI 35K		4694	120 V	41 W		4.
L2E	2X4 RECESSED TO MATCH TYPE L2, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK.	LITHONIA LIGHTING	ENVX 2X4 HRGC 4800LM 80CRI 35K E7W		4694	120 V	41 W		4.
L3	6" DOWNLIGHT	LITHONIA LIGHTING	LDN4 35/15 L04 AR LSS		1492	120 V	18 W		
L3E	6" DOWNLIGHT TO MATCH TYPE L3, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK	LITHONIA LIGHTING	LDN4 35/15 L04 AR LSS EL		1492	120 V	18 W		
L4	2X2 RECESSED	LITHONIA LIGHTING	ENVX 2X2 HRGC 3300LM 80 CRI 35K		3320	120 V	30 W		
L4E	2X2 RECESSED TO MATCH TYPE L4, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK	LITHONIA LIGHTING	ENVX 2X2 HRGC 3300LM 80 CRI 35K E7W		3320	120 V	30 W		
L5	LED STRIP	AXIS LIGHTING INC	BMPRLED-300-90HE-35-RFA-4		1199	120 V	101 W		
L6	LED STRIP	AXIS LIGHTING INC	BMPRLED-300-90HE-35-RFA-4		1199	120 V	27 W		
L7	DECORATIVE SCONCE	BROWNLEE LIGHTING INC	7115-26-H16-35K		1828	120 V	16 W		
L9	LED DOWNLIGHT	LITHONIA LIGHTING	LDN6 35/15 L06AR		1494	120 V	18 W		
L9E	DOWNLIGHT TO MATCH TYPE L9, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK.	LITHONIA LIGHTING	LDN6 35/15 L06AR EL		1494	120 V	18 W		
P	9" ROUND CYLINDER PENDANT	LITHONIA	LDN4CYL 40/20 L04AR LSS MVOLT E21		2000	120 V	23 W		
PE	PENDANT TO MATCH TYPE P, WITH INTEGRAL 90 MINUTE EMERGENCY BATTERY PACK	LITHONIA	LDN4CYL 40/20 L04AR LSS MVOLT E21		2000	120 V	23 W		
R	DECORATIVE PENDANT		CONTRACTOR PROVIDED FIXTURE			120 V	90 W		7.8
R2	DECORATIVE PENDANT		CONTRACTOR PROVIDED FIXTURE			120 V	90 W		7.8
R3	DECORATIVE PENDANT		CONTRACTOR PROVIDED FIXTURE			120 V	90 W		7.8
R4	DECORATIVE PENDANT		CONTRACTOR PROVIDED FIXTURE			120 V	90 W		7.8
S	AREA POLE LIGHT WITH SINGLE HEAD AND TYPICAL MOUNTING OPTICS. BUG RATING: B3-U0-G4.	LITHONIA	DSX0-LED-P7-30K-80CRI-RCC0		12990	120 V	171 W		2.
S2	AREA POLE LIGHT WITH SINGLE HEAD AND RIGHT CORNER CUTOFF OPTICS. BUG RATING: B1-U0-G3.	LITHONIA	DSX0-LED-P7-30K-80CRI-RCC0		12990	120 V	171 W		2.
S3	AREA POLE LIGHT WITH DOUBLE HEAD. BUG RATING: B3-U0-G4	LITHONIA	DSX0-LED-P7-30K-80CRI-T3M		17699	120 V	342 W		5.
WS1	EXTERIOR WALL PACK	BROWNLEE LIGHTING INC	7075-24-TW-H32-35K		3030	120 V	32 W		6.
X1	EXIT SIGN, EDGE-LIT, 1-SIDED	LITHONIA LIGHTING	EXRG			120 V	5 W		1.
SCHEDULE GENERAL NOTES: A. WHETHER INDICATED IN CATALOG NUMBER OR NOT, CONTRACTOR TO PROVIDE ALL NECESSARY ACCESSORIES AND MOUNTING HARDWARE REQUIRED FOR A COMPLETE INSTALLATION. B. COORDINATE FIXTURE COLOR SELECTION WITH ARCHITECT PRIOR TO PURCHASE. C. LEDs SHALL MEET THE FOLLOWING MIN SPECS IN ADDITION TO THE REQUIREMENTS SHOWN ELSEWHERE: a. 50,000 HOUR RATED, MIN CRI = 80, 1 TO 10V DIMMABLE; b. LAMP CCT SHALL CONFORM TO ANSI C78.377A COLOR BINNING AND UTILIZE 4 STEP MACADAM ELLIPSE ALGORITHM BINNING PROCESS. D. EXIT SIGNS WITH BACKWARD LETTERING SHALL NOT BE PERMITTED ANYWHERE. SINGLE-SIDED EXIT SIGNS SHALL HAVE A BLANK OR OPAQUE BACK SIDE AS SPECIFIED BY THE ARCHITECT. DOUBLE-SIDED EXIT SIGNS SHALL HAVE ENGLISH LETTERS "E X I T" THAT READ FROM LEFT-TO-RIGHT ON EACH SIDE. E. LED DRIVERS SHALL MEET THE FOLLOWING MIN SPECS IN ADDITION TO THE REQUIREMENTS SHOWN ELSEWHERE: a. THD LESS THAN 10% POWER FACTOR OR GREATER THAN 90% F. SUBSTITUTIONS ONLY ALLOWED WITH OWNER AND DESIGN TEAM APPROVAL PRIOR TO BID. SUBSTITUTION REQUEST MUST INCLUDE LINE ITEM PRICING BREAKDOWN AND FULL PHOTOMETRIC CALCULATIONS. SCHEDULE NOTES: 1. LED EXIT SIGN, EDGE LIT, MIRRORRED, DIRECTION ARROWS AND MOUNTING AS REQUIRED. PROVIDE WITH SELF-DIAGNOSTICS. 2. FURNISH LIGHT FIXTURE COMPLETE WITH 28" TALL, STRAIGHT, SQUARE STEEL POLE OR APPROVED EQUAL. SECURE TO CONCRETE POLE BASE, PER STRUCTURAL DETAIL. 3. FIXTURES SHALL BE OWNER FURNISHED, CONTRACTOR INSTALLED. 4. MOUNTING AND OPTIONS AS REQUIRED. INSTALL ON BASE PROVIDED BY STRUCTURAL. COORDINATE WITH STRUCTURAL. 5. FURNISH LIGHT FIXTURE COMPLETE WITH 14" TALL, STRAIGHT, SQUARE STEEL POLE OR APPROVED EQUAL. SECURE TO CONCRETE POLE BASE, PER STRUCTURAL DETAIL. 6. PROVIDE FIXTURE VOLTAGE. 7. REFER TO ALLOWANCE #1. 8. CONTRACTOR TO INCLUDE INSTALLATION AND POWER PATH IN THE BASE BID.									

NO	DATE	DESCRIPTION
1	10-16-26	ADD 02